

## RPS, Southern Port Access Road Viaduct, Preliminary Design Report



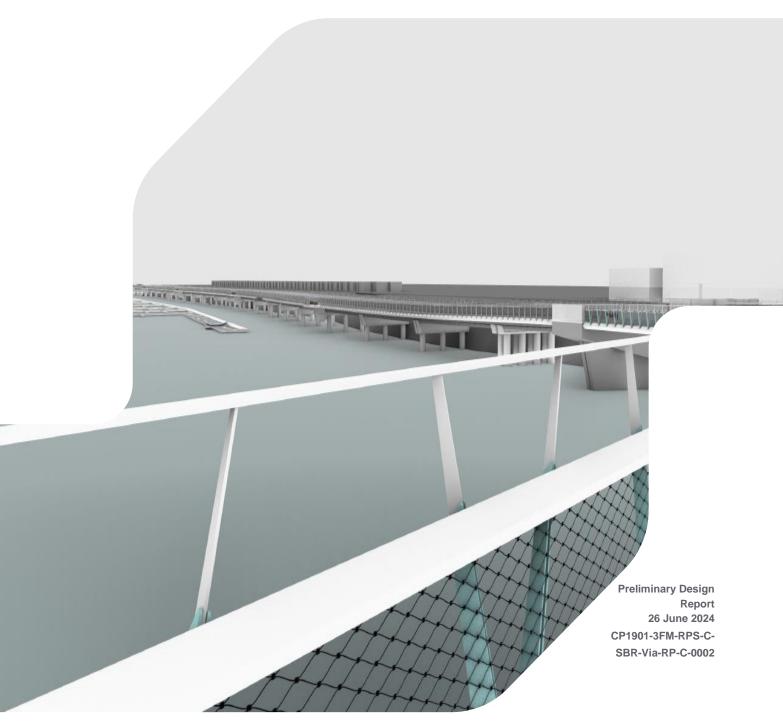






## SOUTHERN PORT ACCESS ROUTE VIADUCT

**Preliminary Design Report** 



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Prepared by:	Prepared for:
RPS	<b>Dublin Port Company</b>

Preliminary Design Report | SPAR Viaduct | 26 June 2024 | CP1901-3FM-RPS-C-SBR-Via-RP-C-0002

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## **Preliminary Design Report - Consultation**

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#### Category 3

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Name and Location: Southern Port Access Route (SPAR) Viaduct

## Structure(s)

Name and nature of the Structure(s): Southern Port Access Route (SPAR) Viaduct

Preliminary Design Report

Reference: IE000336-RPS-00-XX-RP-C-RP0002

Revision: S3 P03

Date: 26/06/2024

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Name:

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Date:

## **EXECUTIVE SUMMARY**

#### **STRUCTURE**

Name: Southern Port Access Route Viaduct

Structure Ref No: TBC

Primary Function: To support the southern port access route from the opening bridge to the west

to the Maritime Village to the east.

Check Category: 3

Loading: IS EN 1991-2 – LM1, LM3 (SV 196), LM4

**PASSAGES** 

Primary: Southern Port Access Route (SPAR)

Secondary: River Liffey

## 1 INTRODUCTION

## 1.1 Instructions or brief given to the authors

This report has been prepared to inform the planning application for the Dublin Port Company's (DPC) 3FM Project. The 3FM project is DPC's third and final masterplan project. The project focuses on developing the Southern Estate of the Port on the Poolbeg Peninsula without which, Dublin Port would reach its maximum capacity limit prior to 2040, which in turn would risk a national port capacity shortage.

## 1.2 Background information covering the origins for the need of the structure

Intensification of the DPC owned lands on the Poolbeg Peninsula is limited by the single carriageway Tom Clarke Bridge. Since conception it has been recognised that the intensification of the Port lands was only feasible if additional road capacity connecting the Southern Estate to the Northern Estate and the M50 (Dublin Tunnel) was provided.

The concept for the Southern Port Access Route (SPAR) was developed in association with and subsequently detached from the Dublin Eastern Bypass (DEB). The NRA initially released a feasibility study in 2007 which included options for the DEB to be evaluated further. The concept for the SPAR developed following the creation and subsequent reviews/revisions of the Dublin Port Masterplan 2040, the Alexandra Basin Redevelopment and further feasibility assessments for the NTA Transport Strategy for the Greater Dublin Area 2022-2042. The SPAR will be a public road with restricted use.

## 1.3 Previous Studies and their recommendations

In 2007, NRA (now TII) released a feasibility study of the Dublin Eastern Bypass (DEB). Sector A Dublin Port to Sandymount had 5 options as shown in Figure 1-1 below taken from Page 47 of the report.

- Option A1 Medium level bridge close to East Link / Tom Clarke Bridge
- Option A2 Shallow Cut & Cover Tunnel close to East Link / Tom Clarke Bridge
- Option A3 Bored Tunnel on most direct route across the central port
- Option A4 High level skew bridge on most direct route across the central port
- Option A5 High level square bridge between central and eastern port areas



Figure 1-1 NRA Dublin Eastern Bypass Feasibility Study - Sector A Route Options

Options A1, A2 and A4 were identified to be brought forward for further evaluation.

Further appraisals were carried out in the intervening period culminating with the introduction of the Southern Port Access Route (SPAR) and the removal of the DEB from Policy. A Route Options for SPAR Working Paper carried out in 2021 identified that the preferred route option for the SPAR would be a medium level bridge close to Tom Clarke Bridge (similar to Option A1 from the 2007 DEB Feasibility Study).

## 1.4 Proposed Development

The proposed road network will primarily consist of a new section of carriageway that will connect the Northern Estate to the Southern Estate, referred to as the Southern Port Access Route 'SPAR'. The SPAR itself is defined as the entire route from North Wall Quay Extension in the north, to the Area O access point at the south, as shown in Figure 1-2 below, with a total length of 2.3km.



Figure 1-2 3FM Project Preliminary General Arrangement Layout (May 2024)

At the northern end, the SPAR will connect into the proposed Berth 18 Access Road which connects to Alexandra Road, providing a congestion free link across the River Liffey on a new bridge and viaduct, landing on the southern shoreline in close proximity to the proposed Maritime Village. The SPAR will then connect into a re-aligned Whitebank Road which connects into the Pigeon Hose Road and South Bank Road. The 3FM Project will also provide upgrades to the existing road network throughout the Southern Port.

### 1.5 SPAR

## 1.5.1 Function (Operational Considerations)

The SPAR will be a public road with restricted use. The SPAR will accommodate port-related traffic om Areas K, L, N and O and connect them to the Northern Estate and the M50 (Dublin Tunnel). Although the majority of SPAR traffic will be HGVs connected with the operation of the Port, the SPAR will also accommodate other traffic flows such as Public Transport, traffic movements from the Encyclis (formerly Covanta) Waste-to-Energy Plant and other Goods vehicles.

The key principles behind the development of the SPAR are:

- The HGV vehicles will be removed from the external DCC road network,
- Port traffic will be relocated further from residential areas,
- Traffic flows will be relatively free-flowing.

These principles should reduce the impact of the HGVs on traffic capacity, congestion, air quality and noise.

#### PRELIMINARY DESIGN REPORT

The SPAR has been developed with an opening bridge section across the River Liffey and a viaduct running alongside the existing R131 until landing on the southern shoreline near the Maritime Village. The bridge section and viaduct will accommodate active travel to provide sustainable transport connections for staff and visitors of the 3FM scheme, in addition to providing a community gain and interconnection between public realm schemes.

## 2 SITE & FUNCTION

#### 2.1 Site location

The SPAR Viaduct is located in east-central Dublin. The viaduct extends from the opening bridge to the west to the proposed Maritime Village to the east over a length of 591.5m. The viaduct is located immediately north of the existing R131 and extends over the southern revetment of the River Liffey east of the existing Tom Clarke Bridge. The SPAR will link the north and south port areas taking Heavy Goods Vehicles (HGVs) off the existing R131.

A location map for the structure is given in Appendix B.

#### 2.2 Function of the structure and obstacles crossed

The SPAR Viaduct is a two-lane carriageway that will traverse the southern revetment/bank of the River Liffey in an east-west direction. The primary aim of the viaduct is to establish a connection between the northern and southern parts of the port, by connecting the SPAR opening bridge to the Maritim Village. The SPAR will play a crucial role in supporting Dublin port economic growth. It is anticipated that a high proportion of HGVs will use this route which will significantly reduce the volume of traffic on the existing R131 and Tom Clarke bridge.

The viaduct will include an active travel facility on the north side of the bridge which will provide pedestrians with a 2.0m walkway, while cyclists will have access to a 3.0m wide two-way cycleway. The active travel facility will connect with the existing active travel routes along the R131 and Poolbeg Peninsula.

Overall, the 3FM project will facilitate efficient transportation of goods and people in the region, while also promoting active travel and reducing traffic congestion on existing routes.

### 2.3 Choice of location

The chosen alignment was determined as a result of studies in CP1901\_3FM-RPS\_26-HGN-XX-RP-C-00001. The tie-in points for the viaduct are dictated by the locations of the southern end span of the SPAR opening bridge to the west and the location and features of the Maritime Village to the east.

## 2.4 Site description and topography

The structure is located in the Dublin Port. The viaduct is located immediately north of the existing R131 and extends over the southern revetment of the River Liffey east of the existing Tom Clarke Bridge. The existing topography for the extents of the viaduct is generally flat.

## 2.5 Vertical and horizontal alignments

The horizontal road alignment is curved to the west to meet the curved end span of the SPAR opening bridge and follows a curved profile to follow the existing revetment/road alignment (up to approximately chainage 1025). From chainage 1025 – 1350 the horizontal alignment is straight until it meets the Maritime Village.

The vertical alignment of the viaduct is straight (flat with no longitudinal gradients) to keep the finished deck levels, parapet levels and barrier levels as low as reasonably possible. This requirement stems from visual impact studies undertaken which studied the impact of the proposed viaduct on river views from existing residences on Pigeon House Road.

## 2.6 Cross sectional dimensions on the alignments

The general cross-sectional dimensions of the viaduct are shown in Table 2-1.

**Table 2-1 Minimum Cross Section of Viaduct** 

Element	Width (m)
Parapet Edge beam	0.5
Walkway	1.951
Cycleway	3.567
Barrier	0.355
Hard shoulder	0.471
Carriageway westbound	4
Carriageway eastbound	4
Hard shoulder	0.471
Barrier	0.355
Maintenance walkway	1.179
Parapet Edge beam	0.5
Total	17.349

## 2.7 Existing underground and overground services

An underground ESB 220kV cable crosses the proposed viaduct location on the eastern side of the bridge. The location was determined from scans. The cable is located within the soft clay layer, between 3-5.0m below ground. The feasibility report (CP1901\_3FM-RPS-S26-HGN-XX-RP-C-00001) discusses services along the SPAR route.

## 2.8 Geotechnical summary

The geotechnical design is discussed in Section 7. Historical boreholes, undertaken in 2011, at the proposed structured location provide information of the anticipated ground conditions. These boreholes indicate a depth of 4-6m of very soft material which would be unsuitable as a founding layer for the viaduct structure.

## 2.9 Hydrology and hydraulic summary

The proposed viaduct will have negligible impact on the hydrology of the River Liffey. The tidal levels at the viaduct are provided in Table 2-2.

Table 2-2 Tidal levels at viaduct

	Highest Astronomical Tide	Mean High Water Spring	Mean High Water Neap	Mean Sea Level	Mean Low Water Neap	Mean Low Water Spring	Lowest Astronomical Tide
	HAT	MHWS	MHWN	MSL	MLWN	MLWS	LAT
	ПАТ		1011 10 01 4	IVIOL	IVILVVIN		LAT
Chart	+4.5m	+4.1m	3.4m	+2.4m	+1.5m	+0.7m	-0.1m
Datum							
(Dublin)							
Ordnance	+1.99m	+1.59m	+0.89m	-0.11m	-1.01m	-1.81m	-2.61m
Datum							
(Malin)							

## 2.10 Archaeological summary

An Environmental Impact Assessment (EIA) is being prepared for the project. It is not expected that the viaduct will have any impact on any Archaeological features in the area.

## 2.11 Environmental summary

The environmental constraints in the vicinity of SPAR viaduct will be determined during the Environmental Impact Assessment process. This report forms part of the basis for the EIA, therefore the conclusions and recommendations of the EIA report will be considered at the detailed design phase of the project.

## 3 STRUCTURE & AESTHETICS

## 3.1 General description of recommended structure and design working life

The viaduct comprises 22 spans of a repeating 27m grid for an overall length of 591.5m. The deck comprises precast prestressed concrete Y4 beams acting compositely with a 200mm thick in-situ reinforced concrete deck slab. Reinforced concrete cantilevers project transversely from the deck to support a steel edge beam and parapet assembly.

The substructure comprises a reinforced concrete crosshead beam supported from discrete projecting piles that are socketed in rock. The cross-head consists of a precast concrete outer shell which is placed on the pile heads and integrated via an in-situ concrete pour to complete the cross-head profile.

The structure is divided into four discrete and separate frames/bays to allow for thermal expansions and contractions. Expansion joints are provided at interface points between adjacent frames/bays. Viewing points and rest points are provided at four intermediate locations along the north of the viaduct.

Solid in-bound precast concrete barriers are bolted to the deck slab to retain errant vehicles and to provide flood protection to a level of +3.95m.

A connection point for active travel is provided from the western end of the viaduct to the existing R131 to the south. The structure supporting the active travel connection comprises a reinforced concrete cantilever slab supported on reinforced concrete bored piles.

The viaduct will have a design life of 120 years. The design life for replaceable structural parts (i.e., waterproofing systems, expansion joints, parapets and safety barriers) shall be 50 years.

The construction of the viaduct is not expected to impact on the R131 revetment sufficiently. However, monitoring of the revetment shall be undertaken during construction.

General arrangement drawings are given in Appendix B.

#### 3.2 Aesthetic considerations

The key consideration for the design of the viaduct is to ensure a smooth transition between the SPAR bridge and the SPAR viaduct and provide a degree of consistency to the journey along the crossing, especially on the active travel path.

#### 3.2.1 Transition between Structures and Break Pier

Both structures have a different structural form – due to the different constraints and demands on the structures – and different widths, meaning there are necessary differences in their cross sections. The transition between the typical SPAR viaduct cross section and the typical SPAR cross section is managed across the Westernmost two spans of the viaduct. The join between the two structures is managed at a shared pier that terminates both viaducts in line with the pedestrian crossing and the active travel link back to land.

The shared pier provides a visual break point that manages the change in structure, extending beyond the parapet on the active travel pathway to express this break point. Either side of the break pier the line of the active travel parapet continues smoothly, as do the active travel pathways. The carriageway also passes smoothly across the two structures. The Western active travel walkway on the SPAR Bridge terminates at the break pier where the active travel path links back to land, as does the Southern maintenance walkway on the SPAR viaduct. On deck there is also a transition between the VRS systems.

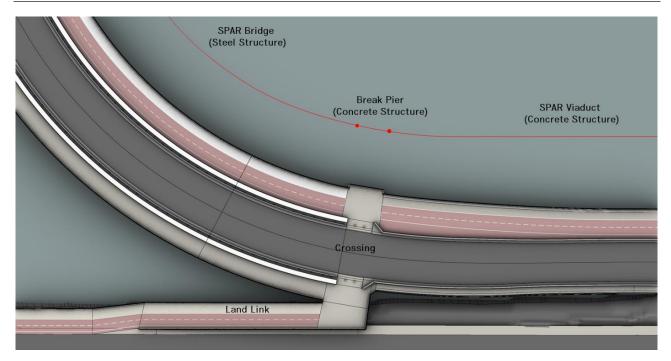


Figure 3-1: Viaduct Transition Plan

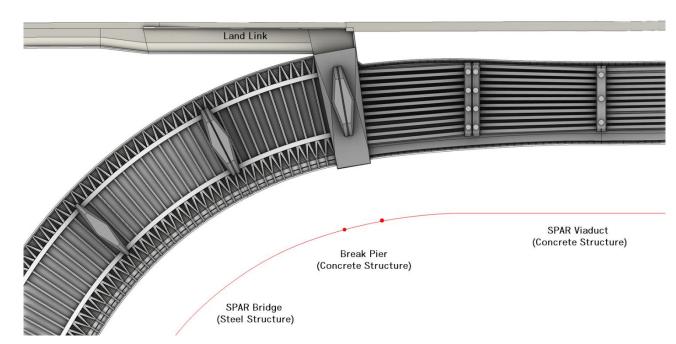


Figure 3-2: Viaduct Transition Soffit Plan

## 3.2.2 Typical Cross Section - Thin Edge & Pedestrian Experience

The proposed viaduct utilises a maximum 3.5m cantilever beneath the Active Travel path which reduces the number of Y-beams required and reduces the width of the substructure. The cantilever provides a thin edge to the bridge that matches that of the SPAR moveable bridge, and puts the structure below in shadow reducing its visual prominence. This narrower substructure results in a reduction of the visual impact of the piles supporting the bridge as well as disguising the petrol separators beneath the cantilever.

The parapet is visually related to those proposed to the SPAR opening bridge to provide a consistent experience for active travel users. The height of the VRS barriers will be limited to 3.950mOD to ensure a view of the River Liffey is maintained for the residents along the southern side of the R131. The concrete VRS barriers provide segregation between the carriageway and active travel pathway. This physical separation between the vehicular traffic and Active Travel users gives a greater feeling of safety and increases the comfort of the bridge users.

Locating the active travel paths on the outside edge of the carriageway allows uninterrupted views off the bridge, allowing views straight down to the water. Physically separating the active travel paths from the carriageway also allows a more lightweight cantilevered structure with slimmer steel edge beams fixed to the concrete structure to be used for the active travel paths. As the paths form the outside elevation of the structure this thin edge reduces the visual impact of the bridge.

Separating the active travel paths from vehicles also opens up opportunities for different surface materials on the paths and allows a lightweight parapet to the outer edges of the bridge. This is especially important as these are the elements that members of the public will come into direct contact with.



Figure 3-3: Diagram showing continuity of edge condition across the SPAR structures

#### 3.2.3 Y-beam Fascia and Pier Crossheads

A pre-cast fascia is provided as part of the external Y-beams to provide shape to the visible face of the beams. The precast concrete cross head shells are shaped to match the fascia and mask the in-situ connections between the beams. The form is repetitive across all piers, with variations to incorporate the double piers at movement joints and house the petrol separators beneath the resting points.

The break pier is a large pre-cast concrete pier with a geometry intended to relate to both the SPAR Viaduct and SPAR Bridge piers.

## 3.2.4 Parapet

As noted in Section 3.2.1, locating the pedestrian walkways outside of the vehicular containment system allows a more lightweight parapet system more suited to a pedestrian environment. Section 3.3.10. describes the technical requirements for the parapet to act as edge protection for the pedestrian and cyclists.

Located on the edges of the bridge the parapet is an important part of the appearance of the bridge from a distance. Simultaneously, as one of the elements closest to the active travel users it is also an important part of the experience from on the bridge.

The proposed parapet consists of a bespoke assembly consisting of an array of twin plate painted steel posts and stainless steel rails with a lightweight stainless steel tension mesh infill. The system is intended to match the parapet system on the SPAR Bridge – albeit with a simplified post geometry – providing a visual and tactile consistency.

The twin-posts support a stainless steel lean rail at 1.1m above deck that encourages pedestrians to pause and take in the views out to sea. The lean rail also conceals lighting for the active travel path.

The tips of the twin posts support a minimal top rail at 1.45m above deck, proposed as a rectangular stainless steel flat. The cross-section of this should be minimised to reduce impact on the views off the bridge.

An infill is required to ensure that there are no gaps in the edge protection through which a sphere >100mm in diameter may pass. The proposed infill is a stainless-steel tension mesh that offers a lightweight infill. The mesh forms a continuous run, broken at the lifting span, and spans vertically between continuous cables fixed to the parapet lugs.

At the break pier wider stainless steel posts frame 3 bays of glazed parapet infill providing a resting point and a visual break between the two related parapet systems on the active travel pathway.

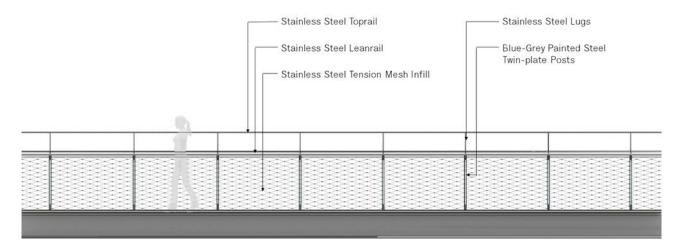


Figure 3-4: Internal Parapet Elevation

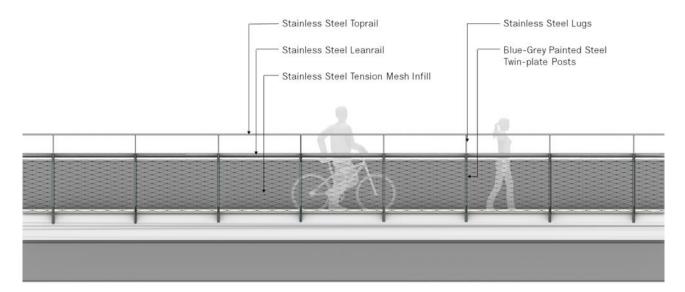


Figure 3-5: External Parapet Elevation

## 3.2.5 Lighting

The functional lighting should be integrated into the elements of the bridge for consistency with the SPAR Bridge. Lighting should be contained within the bridge to avoid excess light pollution or light spilling off the deck into the water and impacting marine life. LED lighting is proposed to provide a compact, low-energy solution.

The proposal to achieve the functional lighting for the active travel pathways is to integrate the luminaires into the parapets. Provision is made within the lean rail for a LED fittings to be mounted discreetly beneath the lean rail. Lighting from beneath the lean rail is intended to wash the walking surface whilst avoiding glare for the bridge users. This provides a consistent solution across the SPAR Bridge and Viaduct.

The functional lighting for the carriageway is proposed to be integrated into the VRS system. Provision is made for linear luminaires to be mounted within the pre-cast concrete VRS barriers

The functional lighting design has been developed in conjunction with RPS Lighting team to verify the low-level lighting solution can meet the required lux levels.

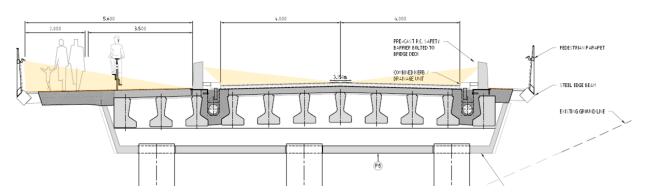


Figure 3-6: Low-level Lighting on Viaduct

## 3.2.6 Surfacing

The SPAR Viaduct surfacing finishes are intended to work with the material palettes established on the SPAR Bridge and the wider active travel scheme. On the main active travel path and the supplementary maintenance path, a grit bounded epoxy combined waterproofing and anti-slip surfacing is proposed. This is applied directly to the structure, offering slip-resistance and a robust finish. Different tones of aggregate will be used for consistency with the wider active travel path surfacing; buff aggregate on the pedestrian walkway and dark red aggregate on the cycleway, white painted lines demarcate the cycle lanes.

The use of aggregate on both pedestrian and cycleway provides is consistent with the wider active travel scheme. The aluminium decking proposed on the SPAR Bridge cannot be used here due to the use of concrete cantilevers on the SPAR viaduct. The buff aggregate finish is proposed over the break-pier also, managing the transition to the finishes on the SPAR Bridge and demarcating the break in the cycleways at the crossing.

At joints in the deck or access hatches stainless steel trims will be provided to contain the aggregate surfacing. The same aggregate surfacing should be provided to the top of hatches for visual consistency.

A strip of stainless steel grating is proposed adjacent to the edge beams on both walkways. This serves to enable access to the fixings for the edge beam for inspection and maintenance, provide drainage to the active travel path through the open slats and highlight the connection to the water below. The grating slats will be laid transversely to the direction of travel, texture to the top of the grating offers slip resistance

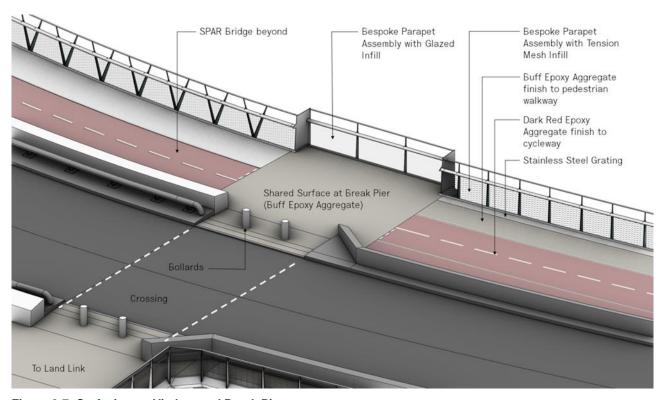


Figure 3-7: Surfacing on Viaduct and Break Pier

## 3.2.7 Resting Points

Resting points are provided at regular intervals along the viaduct. The deck widens at these points providing space to pause without interrupting the flow of pedestrians on the active travel pathway. These locations are co-ordinated with the petrol separators in the piers below – this offers support to the wider deck whilst the extension to the deck also serves to keep the petrol separators in shadow.

The parapet and surfacing at these locations match the typical parapet and active travel walkway surfacing along the rest of the SPAR Viaduct. The resting point geometry and parapet grid shall be coordinated, bespoke corner posts allow the parapet to turn the corner cleanly.

Additional bespoke timber and stainless steel benches are proposed to offer seating. This improves the accessibility of the active travel path by breaking up the length of the journey.

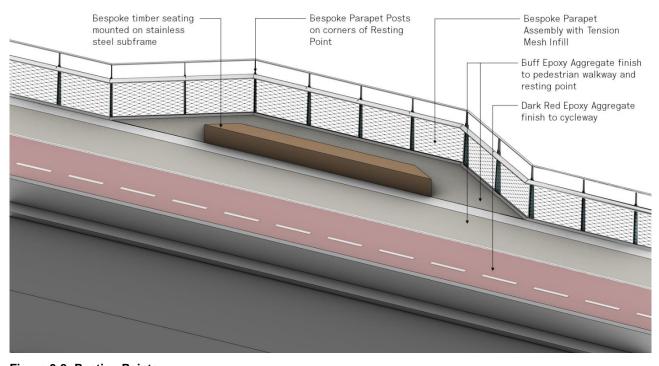


Figure 3-8: Resting Points

### 3.2.8 East Abutment

At the East Abutment the carriageway continues onto land at a low-level, whilst the active travel path separates to meet the public realm at a higher level. This requires the walkway over the final span to diverge from the carriageway alignment as described in the design drawings. The footway rises at a gradient <1:40 to meet the ramps on land at the abutment.

As the active travel pathway raises relative to the carriageway the potential drop on the carriageway side of the VRS increases. The height of the VRS is maintained at 3.950mOD to maintain views to the River Liffey for the residents along the southern side of R131. Therefore, an additional post and rail parapet assembly with a lightweight and visually permeable, tension mesh infill, is introduced to the top of the concrete VRS to provide fall protection. This parapet is a variation of the typical viaduct parapet to provide visual and material consistency.

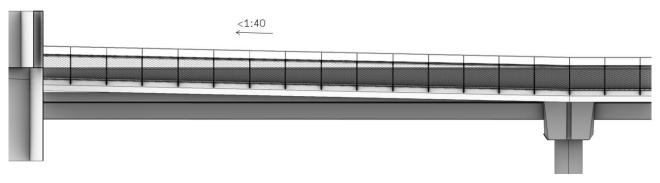


Figure 3-9: Elevation diagram showing footway gently ramp up to meet active travel path at abutment

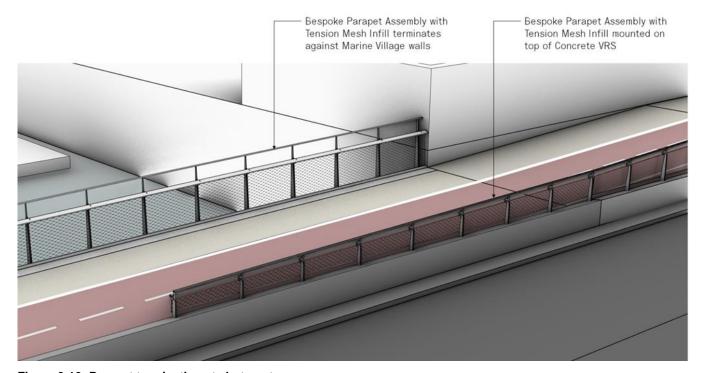


Figure 3-10: Parapet termination at abutment

## 3.2.9 Material Summary

SPAR Viaduct		
Element	Material / Colour	Reference Image
Typical Viaduct Structure	Precast Concrete	

<del></del>	A 1 L	
Carriageway Surfacing	Asphalt	
Carriageway Kerbs	Concrete	
VRS System	Precast Concrete with inset carriageway lighting	
Active Travel Surfacing	Epoxy Aggregate Surfacing in with Stainless Steel trims at joints/access hatches.	See Figures 3-7 & 3-8
	Stainless steel drainage grate a edge of walkway	ıt
Parapet Posts	Blue-Grey Painted Steel	See Figures 3-4 & 3-5
Parapet Rails and Lugs	Stainless Steel	_
Parapet Infill	Stainless Steel Tension Mesh	_
Parapet Infill at Break Pier	Glazed	
Benches	Timber mounted on stainless steel subframe	See Figure 3-8

## 3.3 Proposals for the recommended structure

## 3.3.1 Proposed Category

The proposed structure is Category 3 in accordance with TII DN-STR-03001 clause 3.4.

#### 3.3.2 EN1990 Classes and Levels

In accordance with IS EN1990 Annex B and TII GE-POL-01008 Table A.2 the structure is classified as follows:

Class	Appendix B	Classification	
Consequence Class	Table B1	CC2	
Reliability Class	Table B2	RC2	
Design Supervision Level	Table B4	DSL3	
Inspection Level	Table B5	IL2	

## 3.3.3 Span arrangements

The viaduct comprises 22 spans of a repeating 27m grid for an overall length of 591.5m

## 3.3.4 Minimum headroom provided

The viaduct beam soffit will be at a level of +1.750mOD compared to a Mean Sea Level of -0.11mOD.

## 3.3.5 Approaches including run-on arrangements

The viaduct will connect on the western end to the SPAR opening bridge with a transition span. The road at the eastern end of the structure will be on fill material. It is not proposed to provide run-on slabs to the structure.

## 3.3.6 Foundation type

Based on the geotechnical information, the ground has limited bearing capacity and shallow spread foundations are not feasible. It is proposed to use 1.2m diameter rotary bored piles, with permanent steel casings and a typical length of 33m. The casing will be driven to rock formation. The inside of the steel casing will be rotary bored and have rock sockets. The piles will include a temporary collar to allow placement of the precast crosshead members.

#### 3.3.7 Substructure

The substructure consists of reinforced concrete crosshead beams. The crossheads will be precast concrete shells lifted into position on top of the bored pile collars. Reinforcement and in-situ concrete infill will make the crosshead shells integral with the top of the bored piles and bridge deck.

## 3.3.8 Superstructure

The superstructure comprises precast prestressed concrete Y4 and YE4 beams acting compositely with an in-situ concrete deck slab. FRC permanent formwork spans between beams to support to support the deck slab pour. The beams will be lifted into position on top of the crosshead beams. The deck and diaphragms will be poured in-situ through a defined pour sequence to form an integral structure. Transverse reinforced concrete cantilevers project from the deck edge to support the steel edge beam and parapet assembly at deck edges.

## 3.3.9 Articulation arrangements, joints and bearings

The viaduct consists of four discrete frames with expansion joints located at the interface of each frame. Each discrete frame is fully integral across its spans. The expansion joints are at approximately 165m intervals between discrete frames.

## 3.3.10 Vehicle Restraint System and Pedestrian Parapets

800mm high, precast concrete parapets to DN-REQ-03034 with N2 Containment Level to DN-REQ-03034 are proposed. These will be designed to BS 6779-2 (1991) Highway Parapets for Bridges and Other Structures – Part 2 – Specification for Vehicle Containment Parapets of Concrete Construction.

An opening between the vehicle-resistant system of the viaduct and the SPAR opening is located on the transition to allow for the active travel route from the R131 to the north side of the viaduct. The vehicle restraint system will have flared ends at the opening to mitigate against end-on impact.

The top of the concrete vehicle restraint barrier is 3.950mOD to maintain views of the River Liffey for the residents along the southern side of R131.

At the active travel crossing point, vehicular bollards will be provided to preclude vehicles from accessing the active travel route.

Pedestrian parapets are located on deck edges. The parapet on the northern edge of the structure, adjacent to the active travel facility, will have a minimum height of 1.45m. The parapet on the southern edge of the structure will have a minimum height of 1.25m. The pedestrian parapets will be a bespoke design, and the design will comply with TII DN-STR-03011, TII DN-REQ-03034 & PD CEN/TR 16949:2016. The design loading on the parapet will be combined as defined in PD CEN/TR 16949:2016 Annex A

The pedestrian parapets will comprise painted steel twin plate posts and railings with a lightweight tension mesh infill and will offer visual consistency with the SPAR Bridge as well as a lighter, more open appearance

for the structure for active travel users on the viaduct. The parapet leans out slightly, combined with the mesh infill, making it more challenging for the parapet to climb or achieve a foothold on the parapet

## 3.3.11 Drainage

The vertical alignment of the viaduct is straight (flat with no longitudinal gradients) to keep the finished deck levels, parapet levels and barrier levels as low as reasonably possible. This requirement stems from visual impact studies undertaken which studied the impact of the proposed viaduct on river views from existing residences on Pigeon House Road.

The deck incorporates transverse cross-falls of 2.5% to collect surface water on the carriageway within combined kerb and drainage units located at each edge. The kerb drains will outfall directly to carrier pipes located within drainage troughs provided within the deck profile. The carrier pipes are 300mm diameter and incorporate longitudinal falls of 1 in 350. The drainage system has a capacity for up to a 1 in 100 year storm with 20% climate change. The carrier pipes discharge at regular intervals corresponding to bay ends and outfall transversely at diaphragm locations to petrol interceptors housed within bespoke cross-head profiles.

A transverse fall is incorporated to the transverse deck cantilevers with direct outfalls provided at deck edges through perforated grills in the steel decking.

## 3.3.12 Construction and Buildability

The River Liffey channel will undergo a two stage dredging process. The first stage of the dredging process will dredge the marina area. The second stage of dredging will involve the installation of block stone mattress along the southern side of the channel to protect against undermining of the existing revetment.

Stage 1 of the dredging will be undertaken prior to the construction of both the SPAR opening bridge and the viaduct. Stage 2 dredging includes the installation of the block stone mattress after completion of the substructures.

Restrictions on access from the existing R131 require that construction and logistics is primarily executed from the water via a mix of jack-up barges and floating barges.

It is envisaged that a site compound will be established at the north quay for the purpose of constructing both the viaduct and the opening bridge. The compound will have a plan extents of approximately of 200x75m. Delivery of precast concrete and steel elements to the site will be facilitated through the M50 Port Tunnel. Insitu concrete can be transported to the site through either the M50 Port Tunnel or the concrete batching plant located at South Bank Road. Materials brought to the site compound will be transported by barge to the works area as required minimising construction traffic on the public road system.

It is envisaged that in-situ concrete supply will be provided through either:-

- a. Concrete supply barge using double-pumping methods;
- b. Concrete supply line crossing a temporary jetty at the SPAR opening bridge and onward pumping.

The envisaged construction sequence drawings for the SPAR Viaduct are included in Appendix C. An overview is given in Table 3-1.

**Table 3-1 Construction sequence overview** 

Stage	Work
1.	Stage 1 dredging of marina
2.	Site compound set up at north quay,
	<ul> <li>Set up clear work zone of approximately 3.0m set-back and load spreading measures at north wall quay to limit loading on existing heritage wall during lifting operations</li> </ul>
	<ul> <li>Traffic management set-up as required at north wall quay</li> </ul>

Stage	Work
3.	<ul> <li>Long reach excavator placed on jack up barge</li> <li>Localised temporary works installed at toe of revetment (as required);</li> <li>Jack up barge position at each pier and a long reach excavator will locally excavate toe of revetment to form pocket for subsequent piling</li> <li>Excavate bed locally at each foundation to increase draught (as required).</li> </ul>
4.	<ul> <li>A piling rig will be set-up on a jack-up barge and positioned at each pier,</li> <li>The piling rig will be supported by a crane, concrete and supply barges</li> <li>Once all permanent piles are installed, temporary mooring piles will be installed at each pier location.</li> </ul>
5.	<ul> <li>Installation of the precast cross-head shell on temporary pile collars</li> <li>The cross-head will be fitted with access platforms and edge protection. Gangways to be connected from the top of the revetment for additional access as required.</li> <li>Stage 1 pour to complete cross-head profile</li> </ul>
6.	Complete Stage 2 dredge/stone apron
7.	<ul> <li>Precast beams to be delivered to site compound and lifted on to a supply barge</li> <li>Float precast beams to the crane barge, crane into position on cross-heads</li> <li>Install FRC permanent formwork between beams.</li> </ul>
8.	Pour diaphragms and deck slabs following a defined pour sequence
9.	Cantilever deck edge pour (either in-situ or precast)
10.	Install barriers and parapets
11.	Complete finishes i.e. surfacing and lighting

Table 3-2 provides a preliminary anticipated construction durations for the viaduct.

**Table 3-2 Preliminary construction durations** 

Element	Duration (months)	
Piling	12 months	
Substructure (crossheads/piers)	5 months	
Deck (beams, deck slab & diaphragms)	10 months	
Finishes (parapets, surfacing, verges etc)	2 months	
	Total 29 months	

## 3.3.13 Durability

All elements of the viaduct will satisfy TII publication DN-STR-03012 Design for Durability.

All exposed concrete in the bridge superstructure and substructure will be impregnated with a hydrophobic pore liner in accordance with the TII Specification for Road Works.

Bridge deck waterproofing systems will be spray applied, satisfy the requirements of TII DN-STR-03012 and will be capable of being non-destructively tested. The following surfaces will be protected with a bridge deck waterproofing system:

- The deck slab between parapet upstands;
- The parapet upstands to a height of 100mm minimum above the adjacent deck slab level;
- The back of the abutment from bridge deck level to a level 200mm below the construction joint between superstructure and substructure.
- All buried concrete surfaces apart from those coated with bridge deck waterproofing will be protected
  with two coats of epoxy resin waterproofing. All exposed concrete in the bridge superstructure and
  substructure will be impregnated with a hydrophobic pore liner in accordance with the TII Specification
  for Road Works.

#### Concrete:

The proposed concrete will have minimum exposure class XS3 to the beams and diaphragms.

Concrete strength are as follows:

Piles C40/50
 Diaphragm C40/50
 Deck C40/50

Precast beams

Transfer C35/45Service C50/50

#### Steelwork

Grade S355 W to EN 10025.

The structural steel members shall receive paintwork protection in accordance with TII CC-SPW-0190.

#### Reinforcement:

Reinforcement to be grade 500B or C ribbed bars to BS 4449:2005

Stainless steel reinforcement to be type 1.4301 or 1.4362 to IS EN 10088. Ribbed bars grade 500 to BS 6744:2016 (parapet edge beams). Stainless steel will be used only for links tying the parapet edge beam to the superstructure in accordance with 4.4 of TII DN-STR-03012.

#### Prestressing strands:

Prestressing strands to be 7 wire manufactured from steel Y1860S7 to BS 5896 2012.

## 3.3.14 Sustainability

The primary structural material of the proposed viaduct is reinforced concrete. The concrete material shall largely comprise of 50% Ground Granulated Blast Furnace Slag (GGBS) cement, as it has been proven to possess a lower carbon footprint compared to Ordinary Portland Cement. The proposed structure has been designed to be structurally efficient, thus employing a minimum of materials. At the end of its lifespan, the structure can be readily demolished, and all materials can be recycled.

## 3.3.15 Inspection and Maintenance

The concrete abutments should not require structural maintenance for the design life of the structure. The integral nature of the structure is expected to minimise the maintenance activities throughout the structure.

The configuration of the superstructure has been undertaken to afford good access for inspection and maintenance. Inspection of all components of the soffit of the deck can be done visually by boat from the River Liffey.

The structure will be inspected every 6 years or as required by the TII Eirspan Bridge Management system.

The deck surfacing will need maintenance and replacement after 20 years. Bridge movement joints will need to be inspected and maintained regularly and replaced after 50 years.

## 4 SAFETY

## 4.1 Traffic management during construction including land for temporary diversions

Vehicular or pedestrian traffic management will not be required during the construction of the bridge as the structure will be constructed offline. Minor provision of traffic management at the entrance to the north quay staging area for construction traffic during construction will be required.

There is a possibility of using the transition span as a temporary staging area, where localised traffic management from the R131 may be required for construction traffic.

## 4.2 Safety during construction

In addition to the general obligations and duties under the Safety, Health and Welfare at Work Act 2005, the consultant carrying out the detailed design of the proposed bridge will also undertake the duties of Project Supervisor Design Process (PSDP) and prepare a Preliminary Safety & Health Plan and Designers Risk Assessment for the works. The Designer will take account of the General Principles of Prevention as specified in the First Schedule of the Safety, Health and Welfare at Work (General Application) Regulations and liaise with the Project Supervisor for the Construction Stage (PSCS), as required by the Safety, Health and Welfare at Work (Construction) Regulations 2013.

It is envisaged that the appointed contractor will be experienced in bridge construction of this type and will be appointed PSCS for the duration of the works.

## 4.3 Safety in use

Inspection and maintenance requirements for the structure are minimised due to the structural form chosen. The completed scheme will represent an improvement in safety compared to the existing road conditions.

## 4.4 Lighting

The functional lighting for the active travel pathways will be integrated into the parapet hand rails. The functional lighting for the carriageway is proposed to be integrated within the concrete barrier system. The proposed lighting is similar to the SPAR opening bridge to ensure continuity between the structures.

## 5 DESIGN ASSESSMENT CRITERIA

## 5.1 Actions

#### 5.1.1 Permanent Actions

The material densities and permanent loading will be as per IS EN 1991-1-1 Annex A. The loading of any proprietary system used in the construction of the bridge will be as per the products datasheet and specifications.

Table 6-1: List of Material Weights for Shannon Bridge Crossing

Material	Weight kN/m <sup>3</sup>
Structural steel	78.5
Reinforced Concrete	25.0
Backfill soil	18-20

## 5.1.2 Snow, wind and Thermal Actions

The wind loading at the structure will be designed in accordance with IS EN 1991-1-4.

The structure will be designed for uniform thermal effects in accordance with IS EN 1991-7. Uniform temperature for the bridge location is determined using Figures NA.1 and NA.2 of the Irish national to IS EN 1991-1-5.

The structure will be designed for a vertical differential temperature IS EN1991-1-5 Figure 6.2b. The temperature difference will be considered to act simultaneously with uniform temperature change, as recommended in Irish national Annex, if that is more onerous. The bridge is a Type 2 bridge as per Table 6.1 of IS EN1991-1-5 as such the  $\Delta$ T1 can be interpolated from Table B.2

For road bridges in the normal climatic zone, significant snow loads and traffic loads cannot generally act simultaneously. The effects of the characteristic snow loads on a bridge deck are less onerous than the characteristic value of traffic loads and will not be considered in the design of the bridge.

## 5.1.3 Actions relating to normal traffic

The structure will be designed as per IS EN 1991-2 Load Model 1 (LM1) and Load Model 2 (LM2). The assessment of surcharge for normal loading will be assessed in accordance with IS EN 1991-2 NA.2.36.2. The bridge will be designed for two notional lanes as per IS EN1991-2 clause 4.2.3 table 4.1.

LM1 generally reproduces traffic effects to be taken into account for global and local verifications. The load model is composed of two concentrated axle loads and a uniformly distributed load in each lane. The loading is given in Table 6-2 and the adjustment factor is applied as per the Irish National Annex Clause NA2.12 Table NA.1 of EN1991-2.

Table 6-2: Load Model 1 loading

Location	Tandem system TS Axle loads Q <sub>ik</sub> (kN)	UDL system q <sub>ik</sub> kN/m²	Tandem system TS With adjustment factor to IS NA Axle loads Q <sub>ik</sub> (kN)	UDL system With adjustment factor to IS NA q <sub>ik</sub> kN/m <sup>2</sup>
Lane 1	300	9	300	5.5
Lane 2	200	2.5	200	5.5
Lane 3	100	2.5	100	5.5
Other lanes	0	2.5	0	5.5
Remaining area (qik)	0	2.5	0	5.5

Load Model 2 reproduces traffic effects on short structural members. LM2 consist of a single axle load and is intended for local verification only. The axle load of LM2 is 400kN with dynamic amplification include as per EN1991-2 clause 4.33.

## 5.1.4 Actions relating to abnormal traffic

The structure will be designed for IS EN 1991-2 load models LM3 (SV80, SV100 & SV196). The assessment of surcharge for abnormal loading will be assessed in accordance with IS EN 1991-2 NA.2.36.3. The simplified methods to represent these actions in PD6694-1 (amendment A1 2020) Section 7.6.2 will also be used.

## 5.1.5 Footway or footbridge live loading

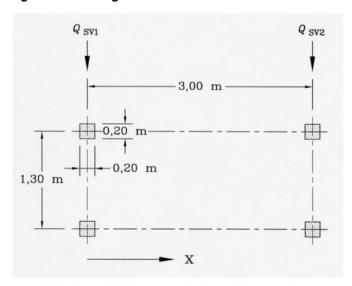
The footway/cycleways will be designed for Load Model 4 (crowd loading) in accordance with Section 4.3.5 and 5.3.1 of I.S. EN 1991-2. This consists of a uniformly distributed load and a concentrated load; these loads are not coexistent.

Table 6-3: Footway Variable loading

EN 1991-2 Clause	Loading type	Load
5.3.2.1(1) and NA2.38	Vertical uniformly distributed variable load q,fk	5.0kN/m <sup>2</sup>
5.3.2.2(1) and NA2.39	Vertical concentrated variable load Q,fk	20 kN over a 0.2m x 0.2m patch
5.4.(2)	Horizontal variable load Q,fk	10% of total q <sub>n</sub>

The foot/cycle will be design to for the accidental vehicle of foot bridge as given by FIGXX. This vehicle is to represent a service vehicle on the active travel route. The vehicle consist of two axles, the front axle of 40kN and the back axle of 80kN, a wheel base 3.0m, and a wheel centre to centre of 1.3m. A 60% braking force will be associated with the vehicular load. No other variable load will accompany the vehicle on any associated span.

Figure 5-1 Accidental loading/service loading



Key

x : Bridge axis direction

 $Q_{\text{sv1}} = 80 \text{ kN}$ 

 $Q_{\text{sv2}} = 40 \text{ kN}$ 

## 5.1.6 Provision for exceptional abnormal loads

Not applicable

#### 5.1.7 Accidental actions

Collision forces on the bridge parapet will be considered in accordance with IS EN 1991-2. Parapet supporting structures will be designed for accidental load effects corresponding to 1.25 times the characteristic resistance of the vehicular parapet system.

## 5.1.8 Actions during construction

The structure will be designed for the construction loading in accordance with EN 1991-1-6 table 4.1. The construction loading will be based on the envisaged construction sequence given in Section 3.3.11. The construction sequence will be further developed during detail design with appropriate allowances for temporary works based on the developed construction methodology. These allowances will be noted on the drawings.

## 5.1.9 Any special loading not covered above

None proposed.

#### 5.1.9.1.1 Grouping of traffic loading

The characteristic values of traffic actions acting simultaneously with non-traffic action are defined by the five Group Models in EN1991-2 Table NA.3 of IS EN 1991-2. It is proposed to design the bridge for the following group models:

Table 6-4: Assessment of Group Traffic Loads

Load		Carriageway			Footways and Cycle Tracks
		Vertical forces		Horizontal forces	Vertical forces only a
References		4.3.2	Annex A	4.4.1	5.3.2.1 Equation (5.1)
Load System		LM1 (TS and UDL)	LM3 (Special vehicles)	Braking and acceleration forces	Uniformly distributed load
Groups of loads	gr1a	Characteristic			0,6 times Characteristic
	gr5	Frequent	Characteristic		
	gr6		Characteristic	Characteristic	
		Dominant component action			

## 5.2 Authorities consulted and any special conditions required.

The following authorities were consulted during the conceptual design phase:

Dublin Port Company

#### PRELIMINARY DESIGN REPORT

- Dublin City Council,
- TII,
- OPW
- ESB

## **5.3** Proposed Departures from Standards

None proposed.

## 5.4 Proposed methods of dealing with aspects not covered by Standards

None proposed.

## 6 GROUND CONDITIONS

## 6.1 Geotechnical Classification

The geotechnical classification of the proposed structure is Category 2 as per BS EN 1997-1.

## 6.2 Description of the ground conditions and compatibility with proposed foundation design

A Geotechnical Design Report (GDR) is being prepared. Historical boreholes were carried out around the location of the proposed structure in 2011 and this information provides an anticipation of the ground conditions of the site. A summary and approximation of the information obtained from the boreholes in the vicinity of the proposed structure are shown in Table 7-1. The bord log are provided in Appendix D.

A general description of the ground conditions encountered in the historical boreholes is detailed below, in Table 7-1.

**Table 7-1: Summary of Ground Conditions** 

Material Type	Depth to top of unit (m bgl)	t Range of unit thickness (m)	Typical Description	Material Type
Pavement – bound layers	0	0.06 to 0.50	Bitmac, concrete and paving stones. At some locations, paved surfacing was underlain by a second concrete/bitmac layer	Pavement – bound layers
Made Ground – Sub-base	0.06 to 0.50	0.30 to 1.20	Sandy rounded fine to coarse GRAVELS with medium cobble content and low boulder content overlying gravelly silty fine to coarse SANDS with low cobble content and fragments of construction materials or gravelly sandy SILTS with fragments of construction materials	
Made Ground – Fill	0.15 to 0.50	2.50 to 6.80*	Reworked sandy gravelly CLAY/ SILT fill or sandy silty/clayey GRAVEL or gravelly silty/ clayey SAND fill with varying amounts of concrete, red brick, timber, steel and glass fragments as well as varying amounts of wire, plastic, cloth, and ash	Made Ground – Fill
Marine Beach Deposits	2.50 to 6.80	11.20 to 15.10	Medium dense to dense SANDS and GRAVELS interspersed with layers of sandy gravelly CLAY frequently with shell fragments encountered across the site to a maximum depth of 20.10m in	Marine Beach Deposits
Port Clay	15.00 to 20.10	10.40 to 14.75	Firm to stiff sandy silty CLAY often with laminations of silty sand	Port Clay
Glacial Till / Fluvioglacial deposits	26.80 to 36.50	3.00 to 11.45	Very stiff sandy gravelly CLAY or very dense sandy clayey GRAVEL	Glacial Till / Fluvioglacial deposits

<sup>\*</sup>Note – A depth of Made Ground – Fill to a depth of 15.80m was encountered in BH130 and a depth of 15.40m in BH131. This location is through an existing caisson, and aside from this the maximum depth was 6.80m in the south of the site, which is a former landfill area.

## 7 DRAWINGS AND DOCUMENTS

## 7.1 List of all documents accompanying the submission

Table 8-1 lists drawings are included in Appendix B of the report.

Table 8-1: Drawing List

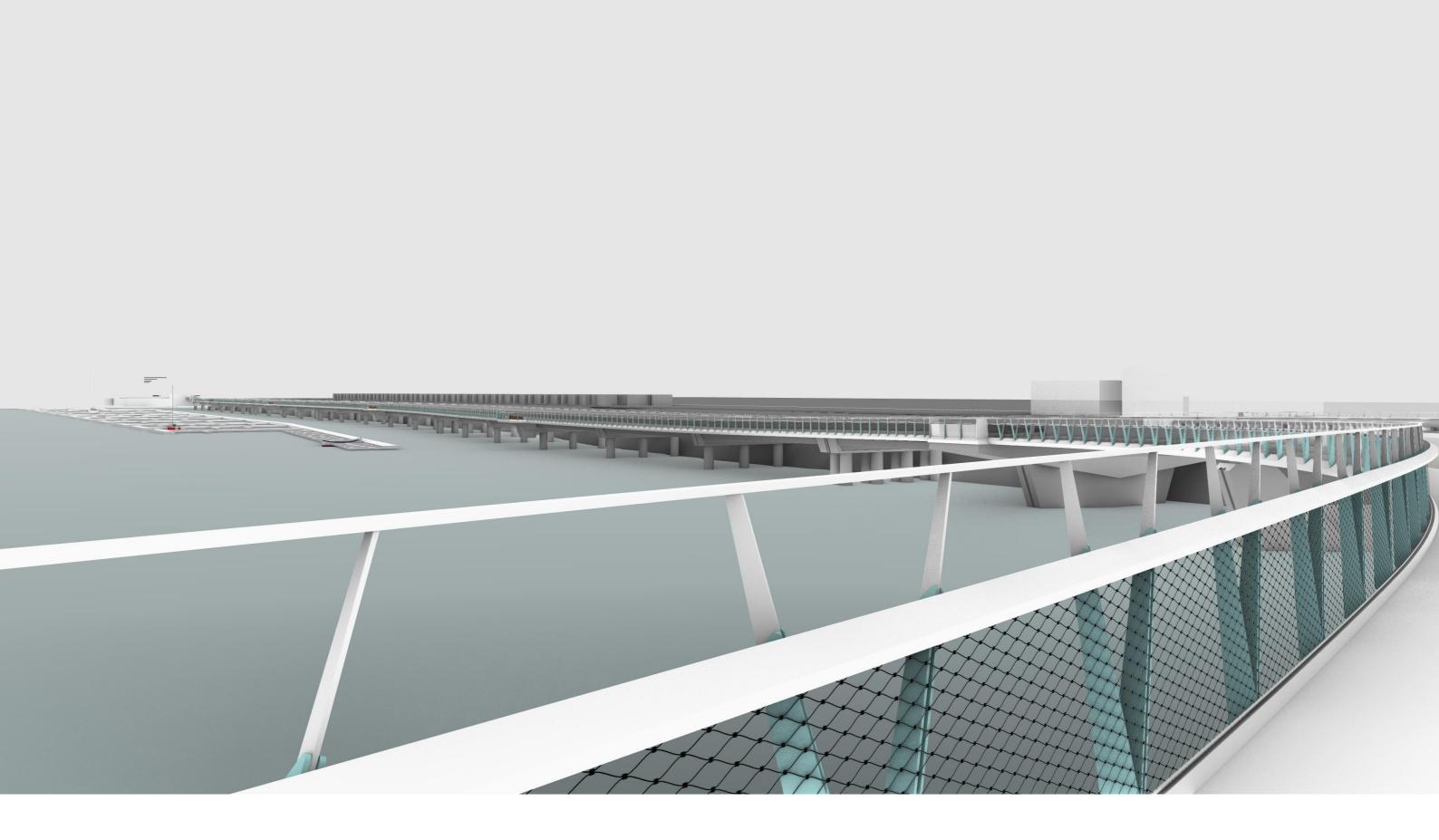
Drawing No.	Status	Rev.	Title
CP1901-3FM-RPS-C-SBR-Via-RP-C 0102-01	S3	P02	SPAR-Proposed Viaduct Location
			Plan (sheet 1 of 1)
CP1901-3FM-RPS-C-SBR-Via-RP-C—0103- 01	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 1 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-02	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 2 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-03	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 3 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-103-04	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 4 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-05	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 5 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-06	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 6 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-07	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 7 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C -0103-08	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 8 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-09	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 9 of 10)
CP1901-3FM-RPS-C-SBR-Via-RP-C-0103-10	S3	P02	SPAR-Proposed Viaduct General
			Arrangement (sheet 10 of 10)

## 7.2 Documents

The Appendices to this report are:

- Appendix A Photographs and photomontages
- Appendix B General Arrangement Drawings
- Appendix C Construction sequence drawings
- Appendix D Geotechnical Information

# Appendix A Photographs and photomontages





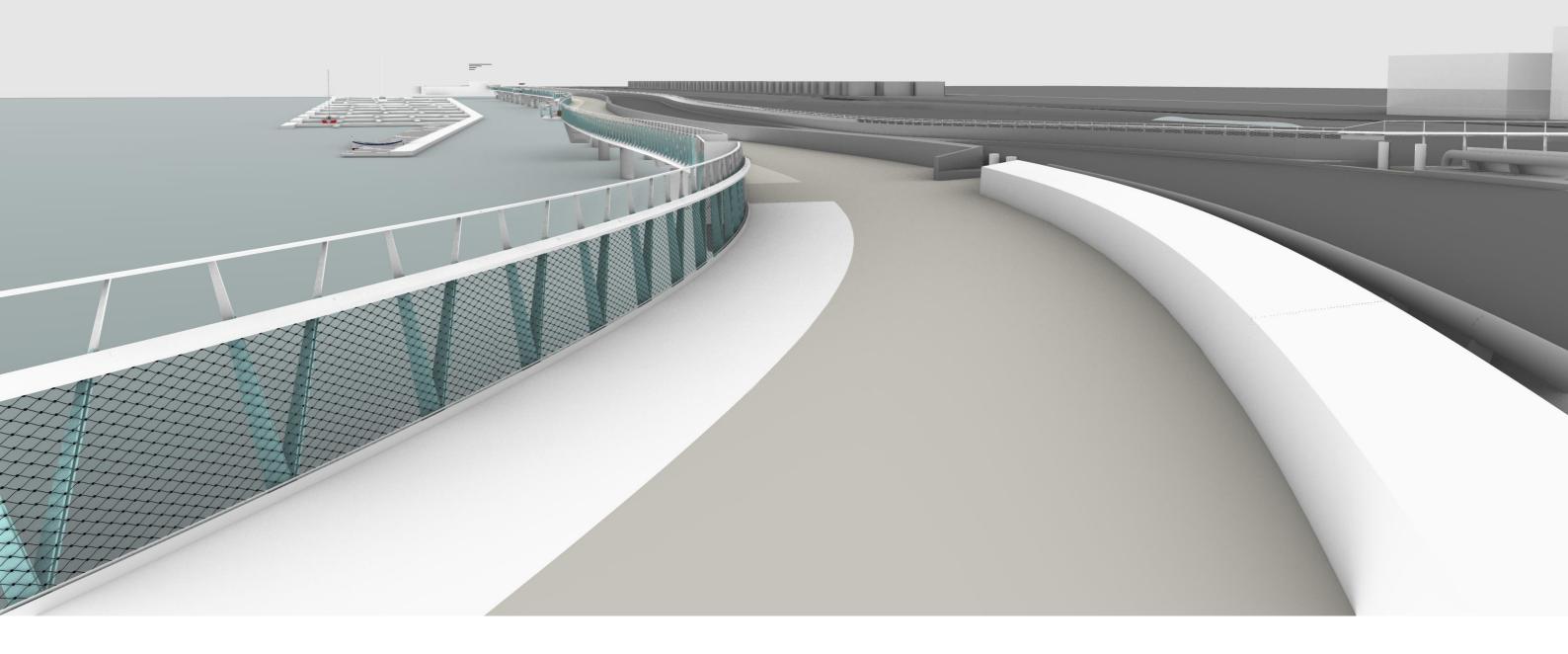






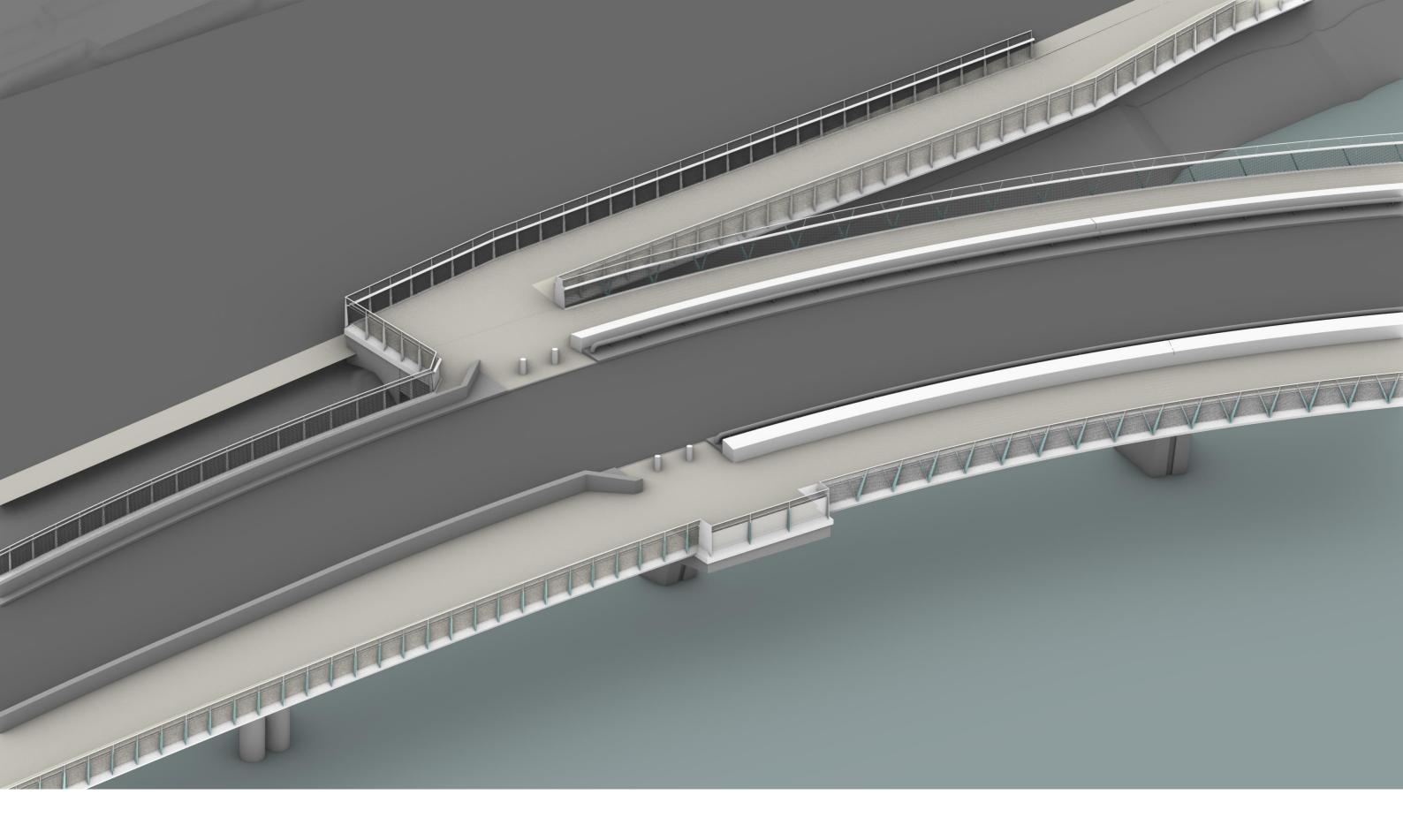








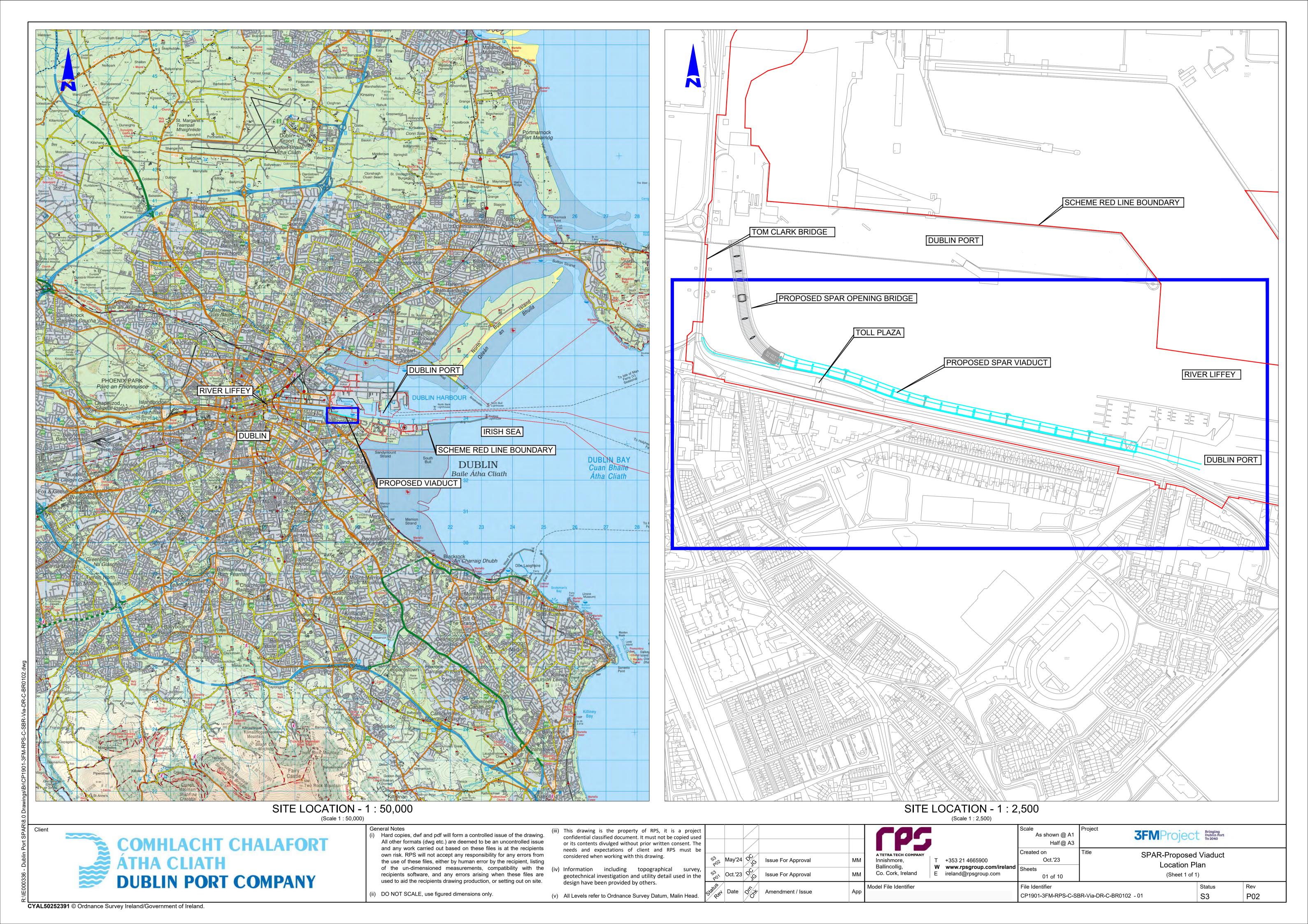


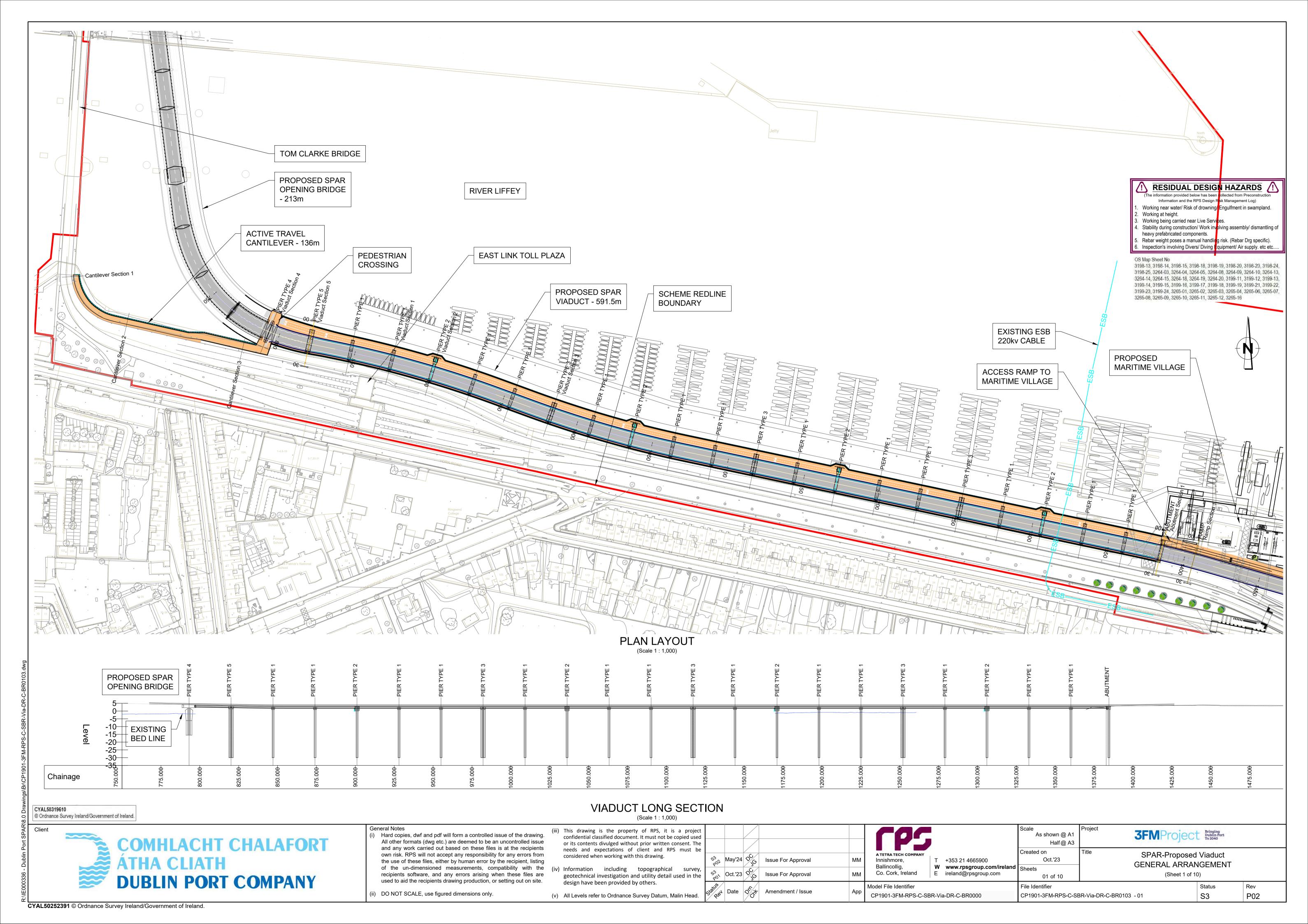


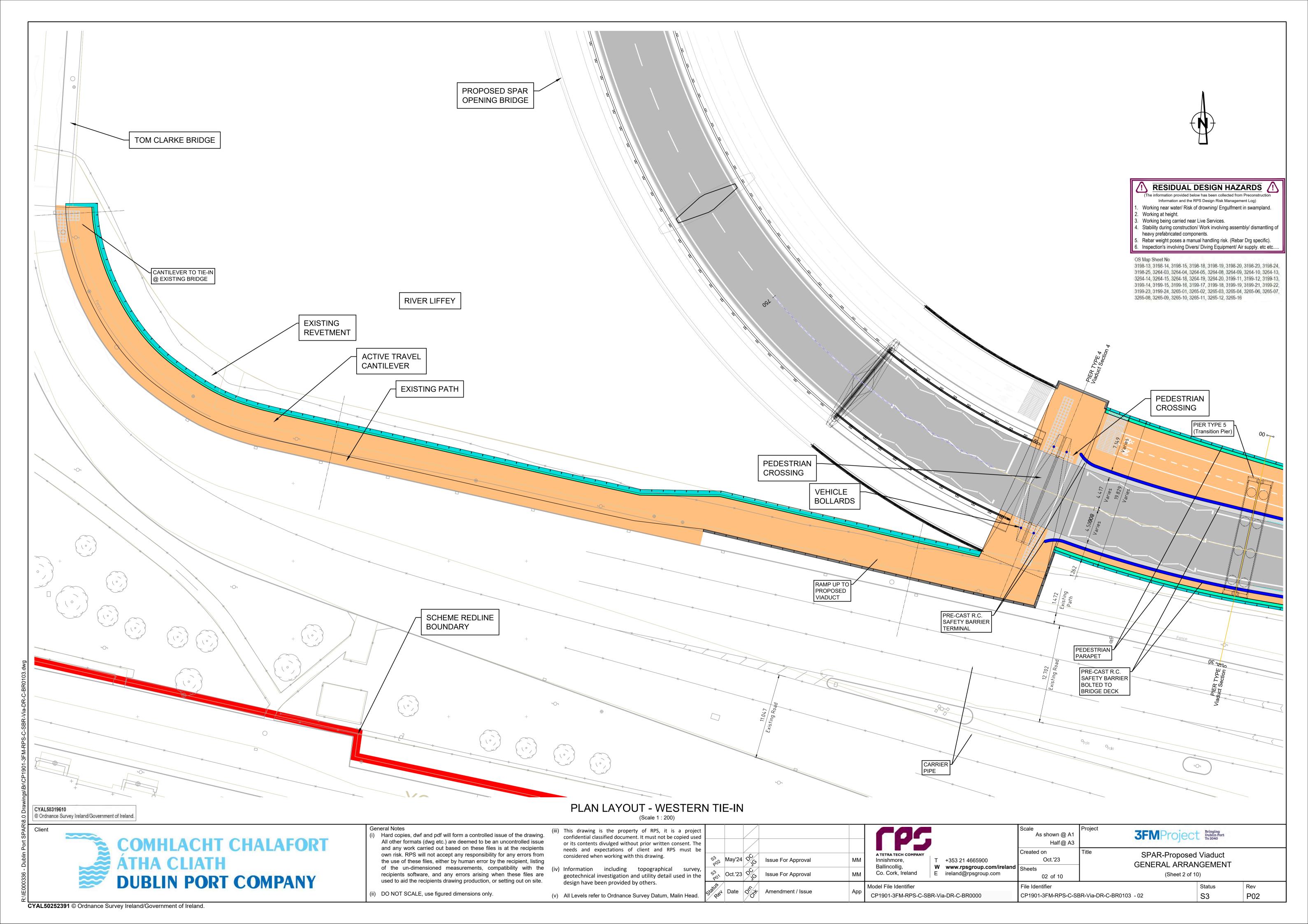


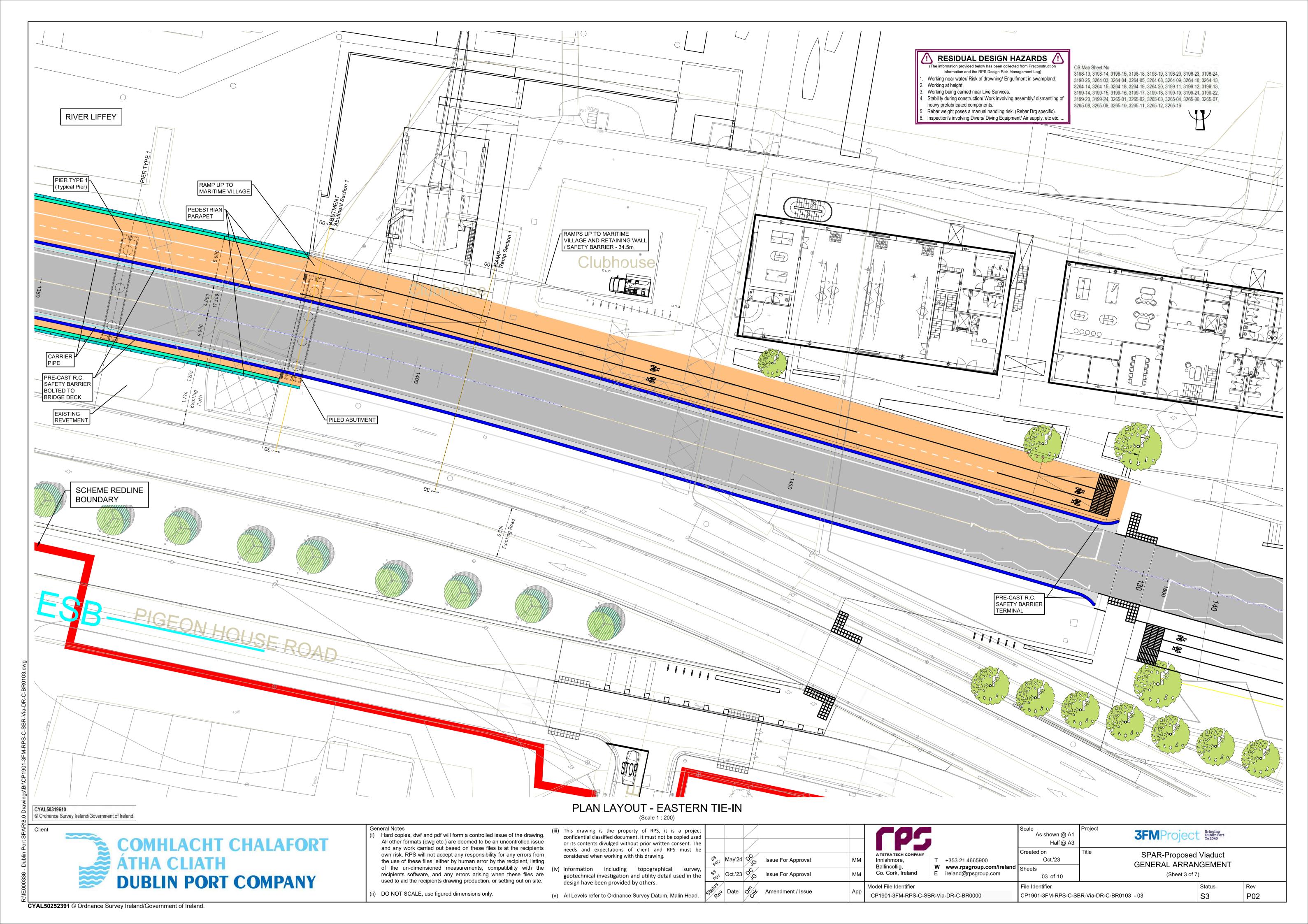


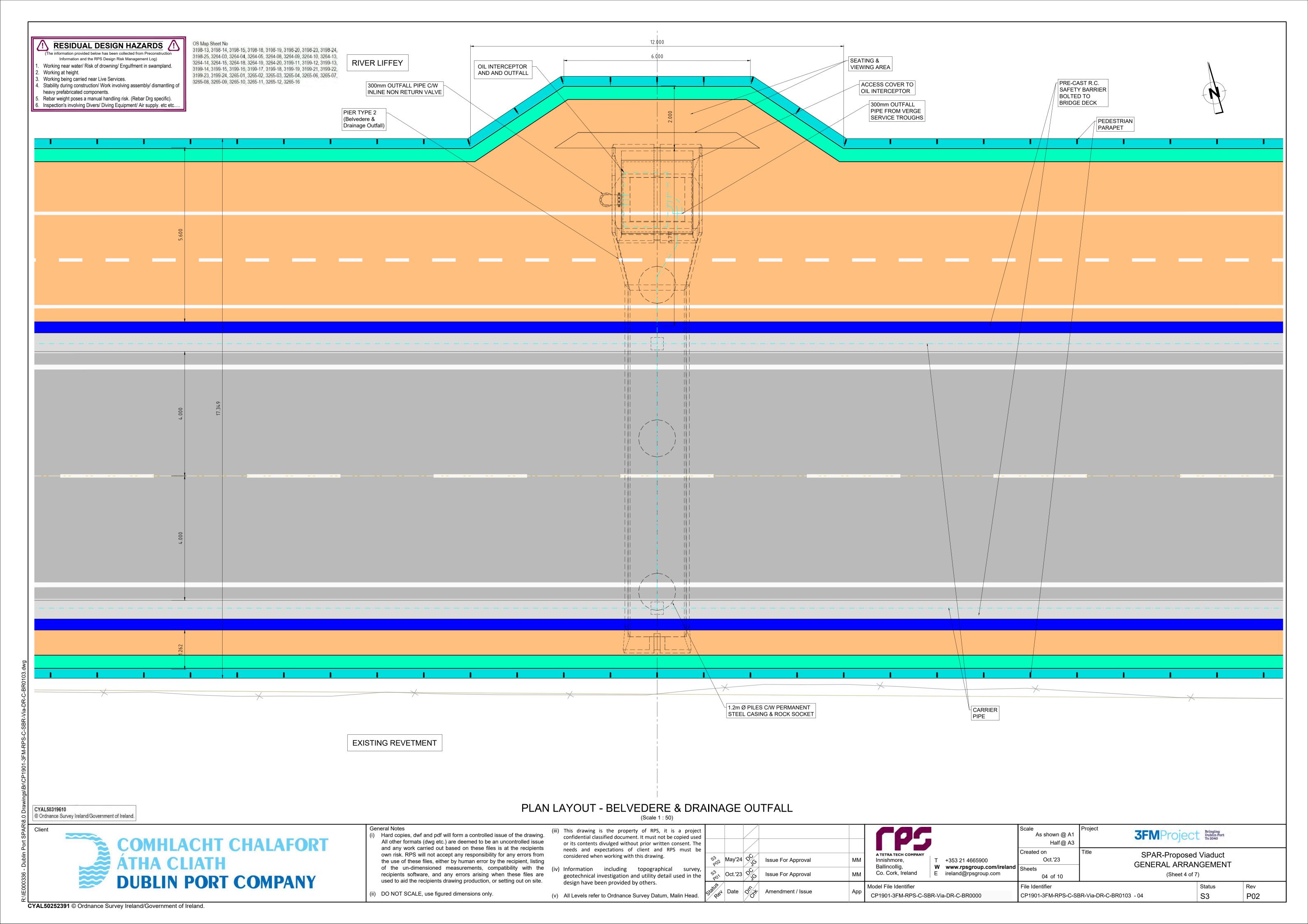
## **Appendix B General Arrangement Drawings**

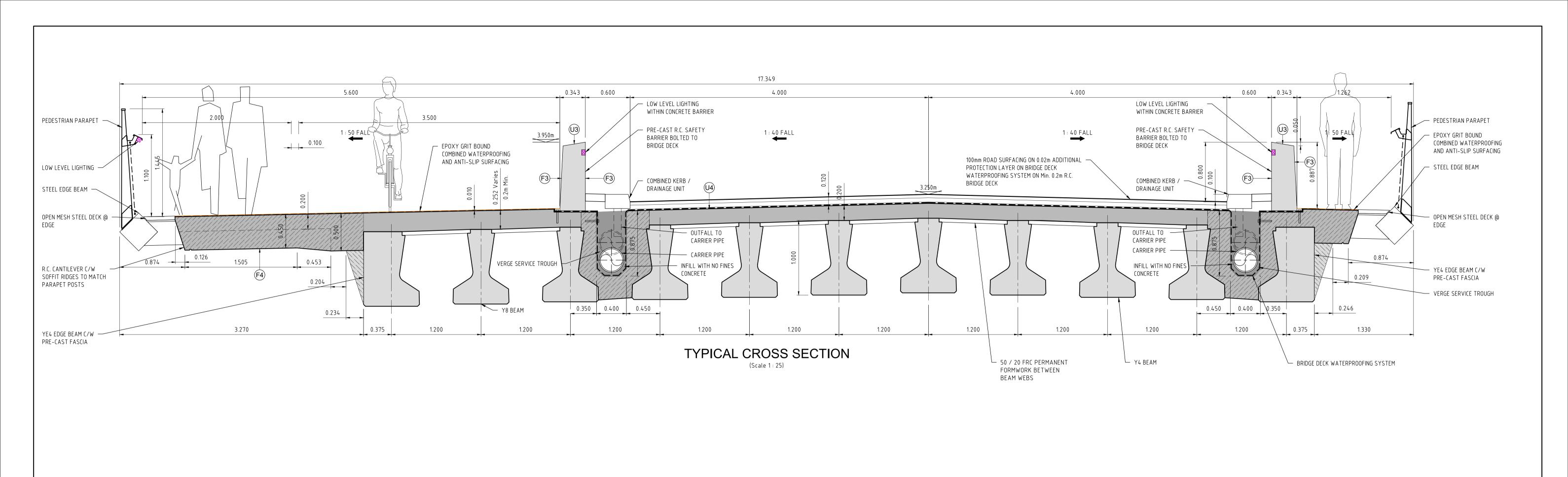


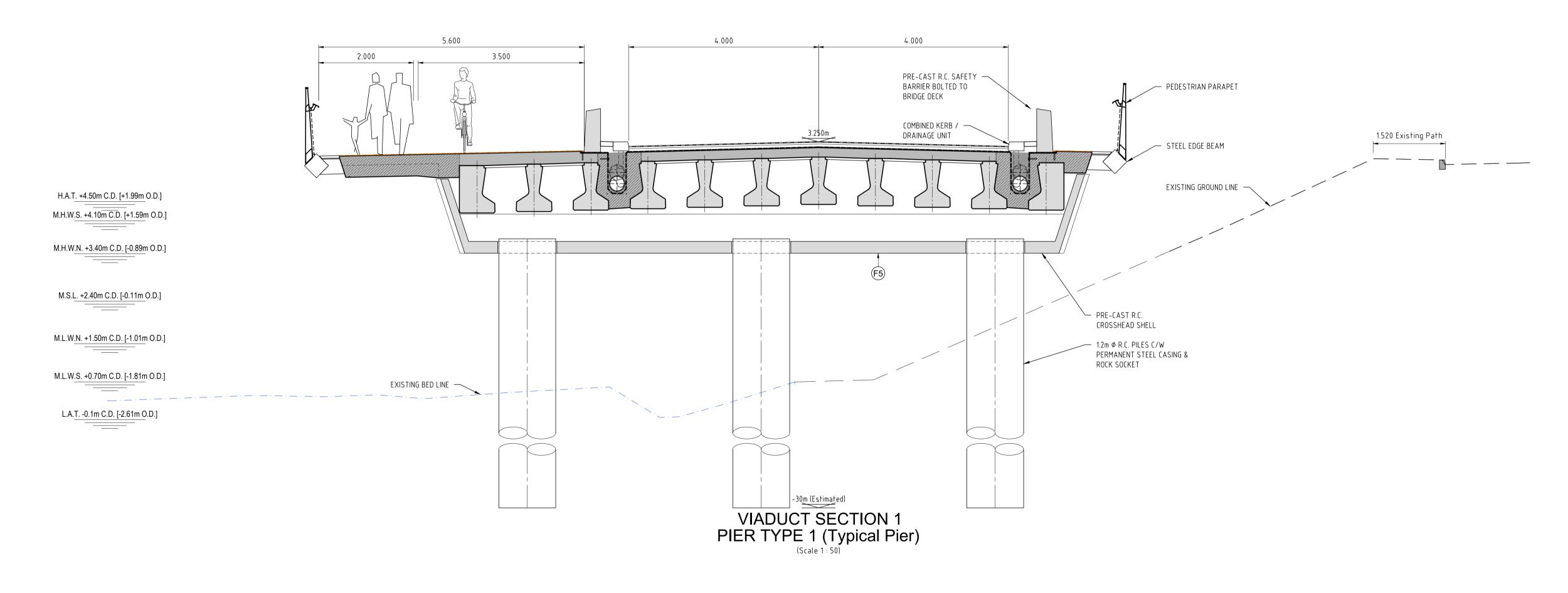












RESIDUAL DESIGN HAZARDS (1)

Information and the RPS Design Risk Management Log)

Working near water/ Risk of drowning/ Engulfment in swampland.

Working at height.

6. Inspection's involving Divers/ Diving Equipment/ Air supply. etc etc...

S3

Working being carried near Live Services.

. Stability during construction/ Work involving assembly/ dismantling of heavy prefabricated components. Rebar weight poses a manual handling risk. (Rebar Drg specific).

Rev

P02

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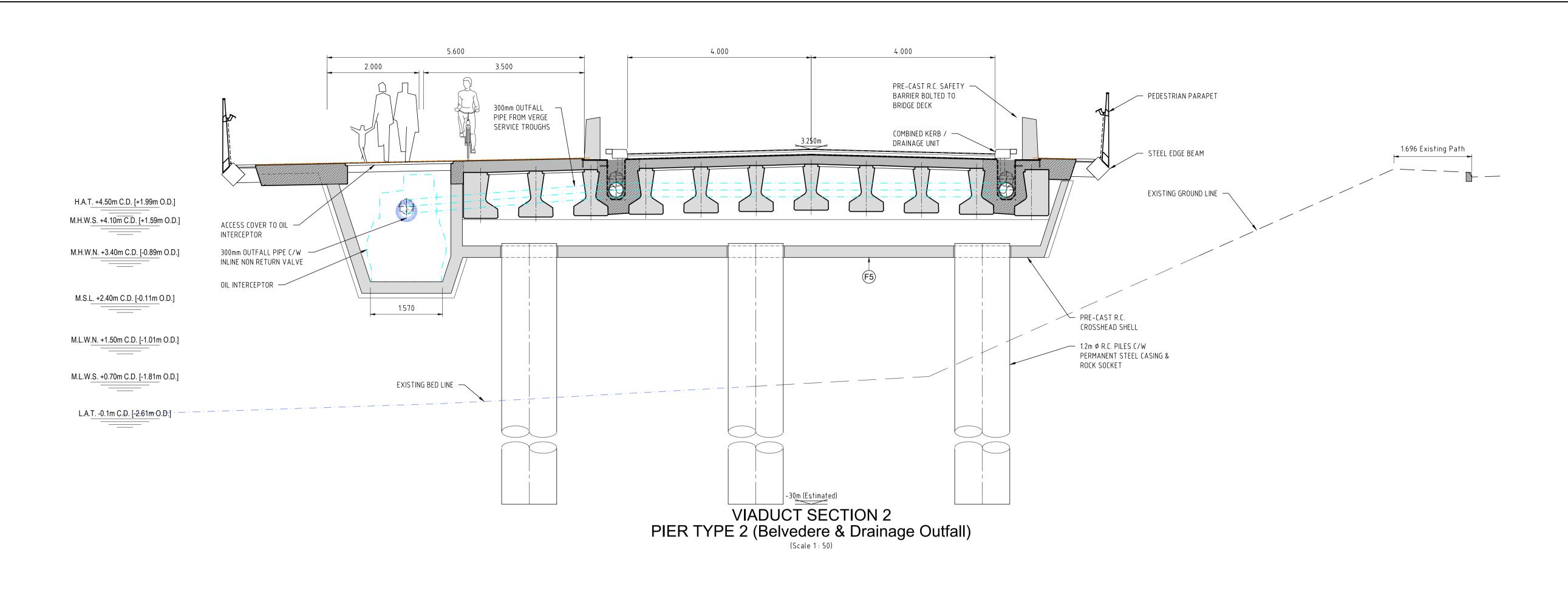
(v) All Levels refer to Ordnance Survey Datum, Malin Head.

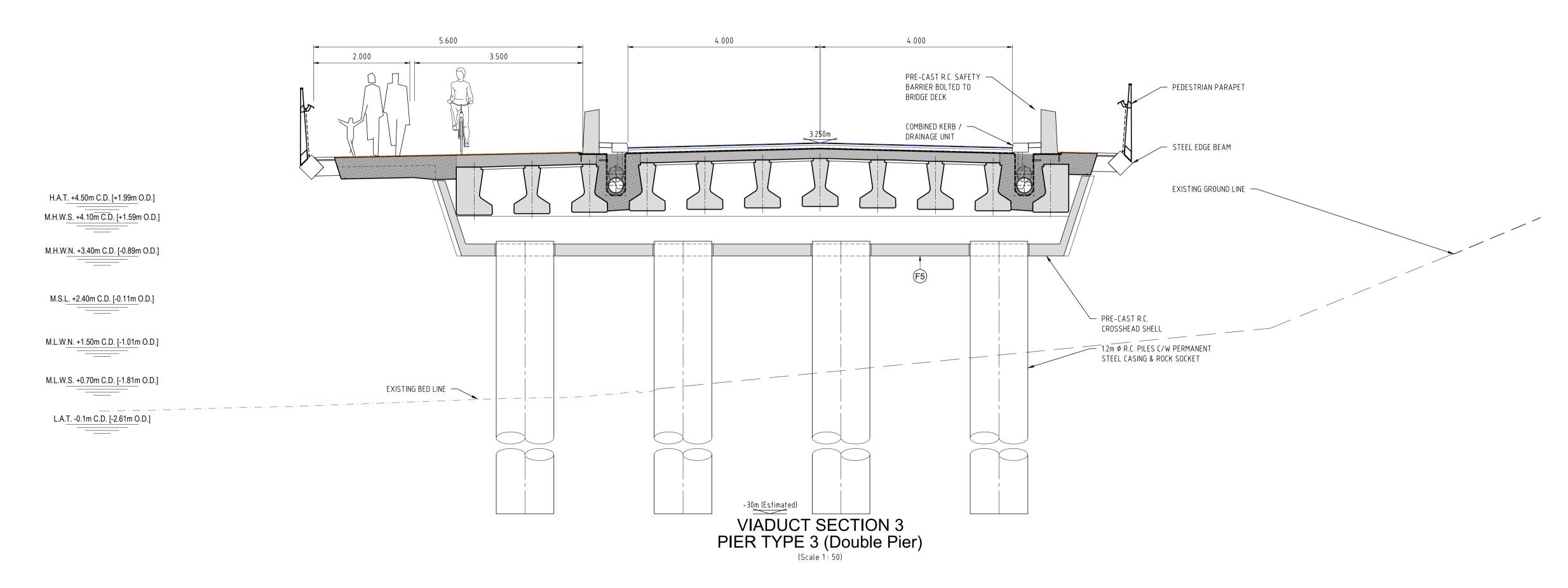
design have been provided by others.

May'24 S Issue For Approval Issue For Approval geotechnical investigation and utility detail used in the Date | State | Amendment / Issue

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RESIDUAL DESIGN HAZARDS (1)

Information and the RPS Design Risk Management Log)

Working near water/ Risk of drowning/ Engulfment in swampland.

Working at height.

Working being carried near Live Services.

Stability during construction/ Work involving assembly/ dismantling of

heavy prefabricated components.

. Rebar weight poses a manual handling risk. (Rebar Drg specific).

6. Inspection's involving Divers/ Diving Equipment/ Air supply. etc etc....

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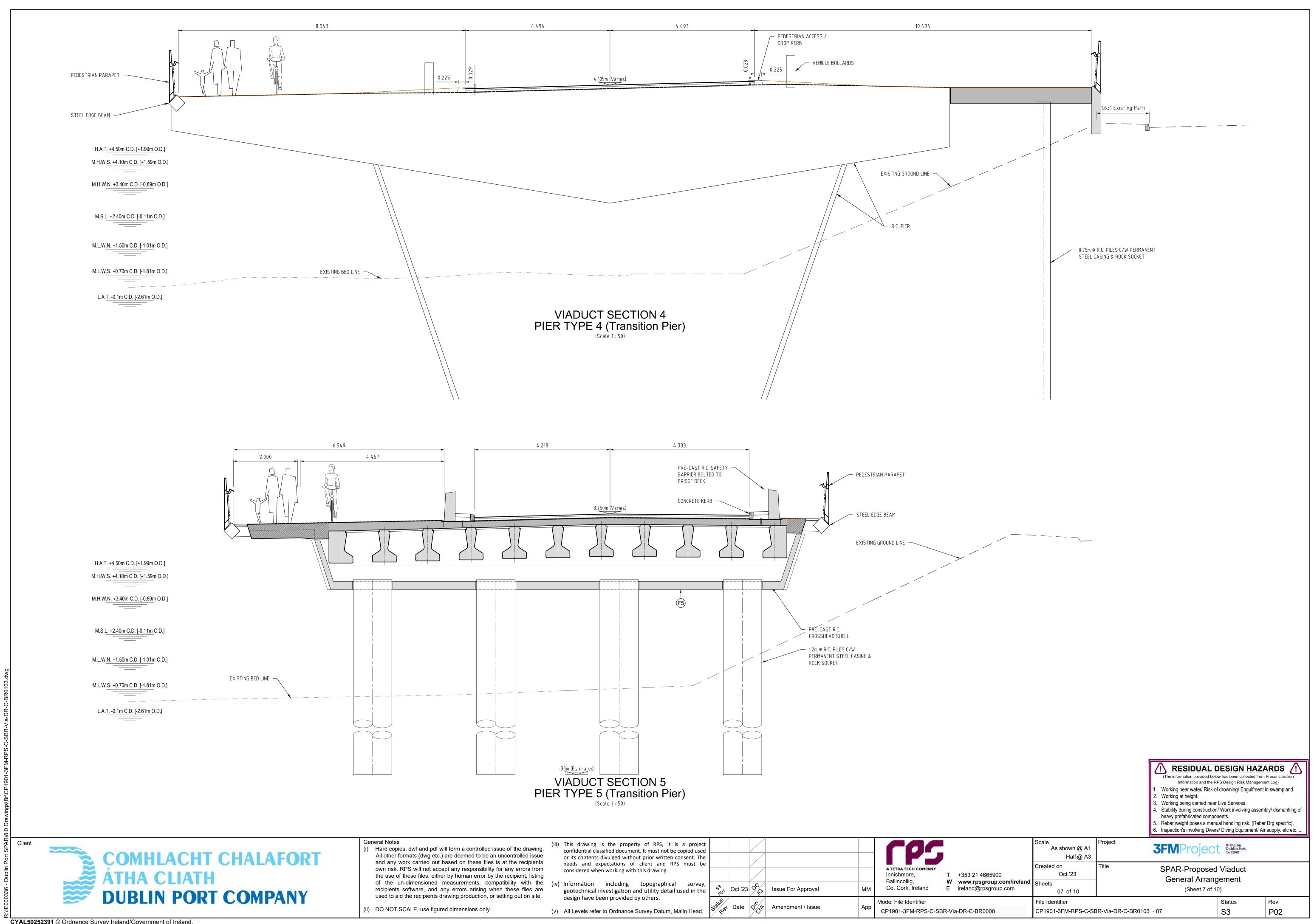
design have been provided by others.

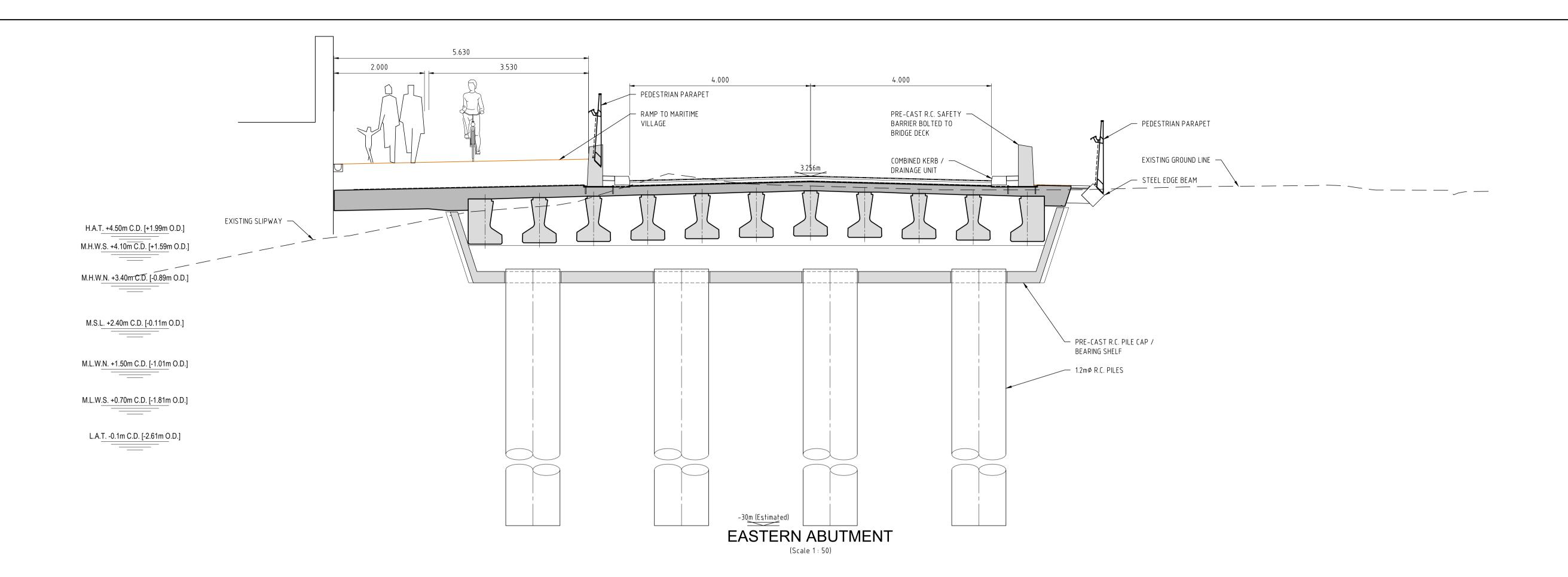
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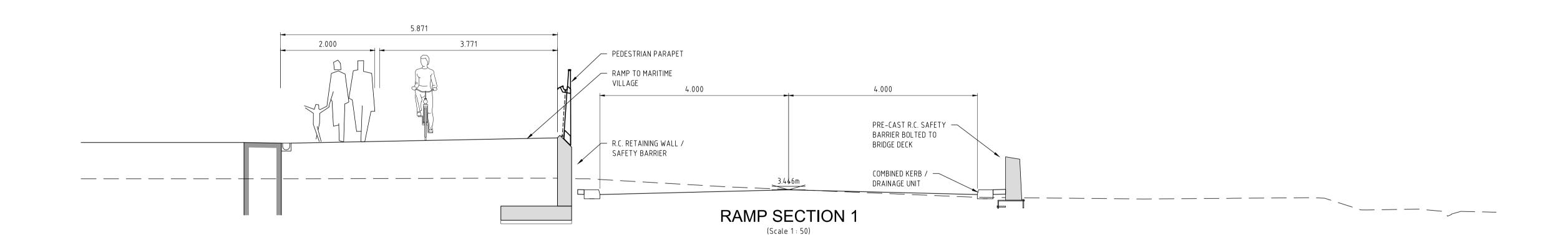
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Rev CP1901-3FM-RPS-C-SBR-Via-DR-C-BR0103 - 06 S3 P02







RESIDUAL DESIGN HAZARDS (1)

Information and the RPS Design Risk Management Log)

Working near water/ Risk of drowning/ Engulfment in swampland.

. Working at height.

Working being carried near Live Services.

Stability during construction/ Work involving assembly/ dismantling of heavy prefabricated components.

5. Rebar weight poses a manual handling risk. (Rebar Drg specific).6. Inspection's involving Divers/ Diving Equipment/ Air supply. etc etc.....



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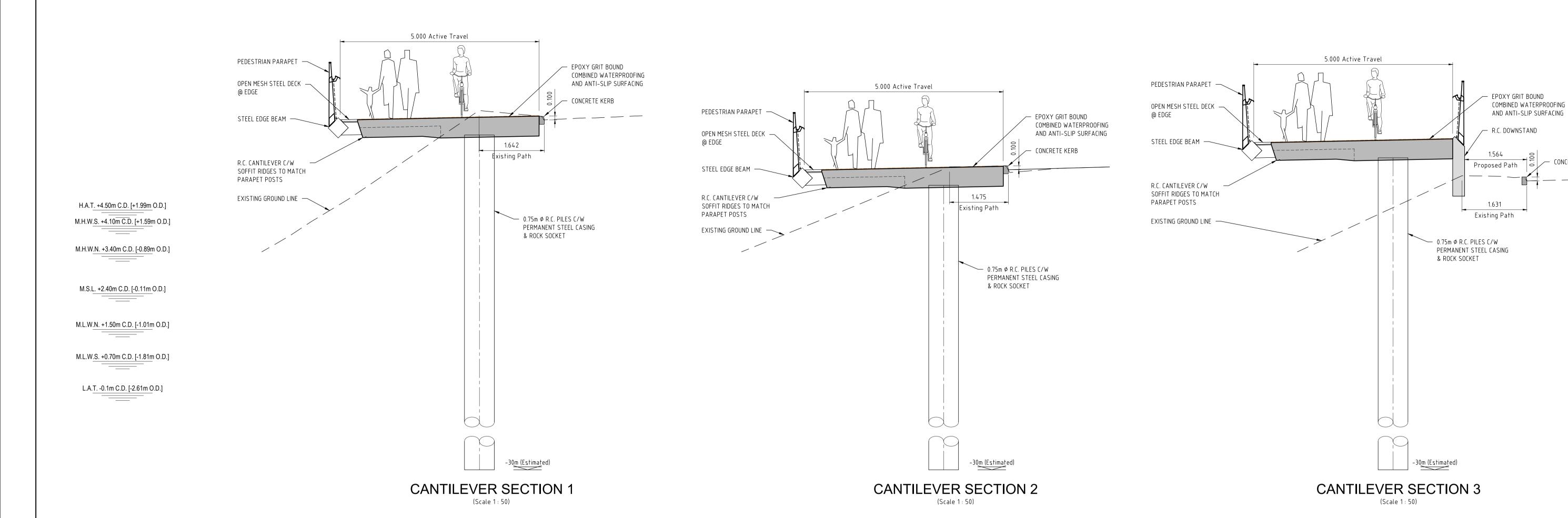
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- Information and the RPS Design Risk Management Log) Working near water/ Risk of drowning/ Engulfment in swampland.
- Working at height.
- Working being carried near Live Services. 4. Stability during construction/ Work involving assembly/ dismantling of
- heavy prefabricated components. 5. Rebar weight poses a manual handling risk. (Rebar Drg specific).

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6. Inspection's involving Divers/ Diving Equipment/ Air supply. etc etc....

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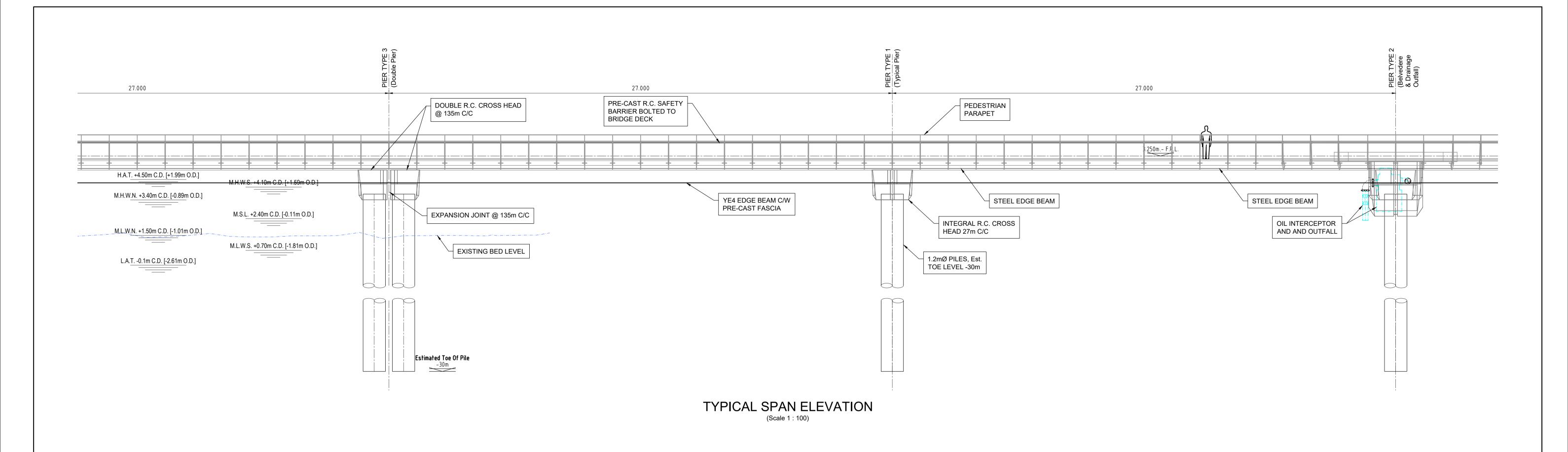
or its contents divulged without prior written consent. The May'24 O Issue For Approval Solution Oct.'23 Oct.'23 Oct.'23 Oct.'23 Date State Amendment / Issue (v) All Levels refer to Ordnance Survey Datum, Malin Head. CP1901-3FM-RPS-C-SBR-Via-DR-C-BR0000

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(Sheet 9 of 10) S3 P02





- Information and the RPS Design Risk Management Log)
- Working near water/ Risk of drowning/ Engulfment in swampland.
- . Working at height.
- . Working being carried near Live Services. 4. Stability during construction/ Work involving assembly/ dismantling of
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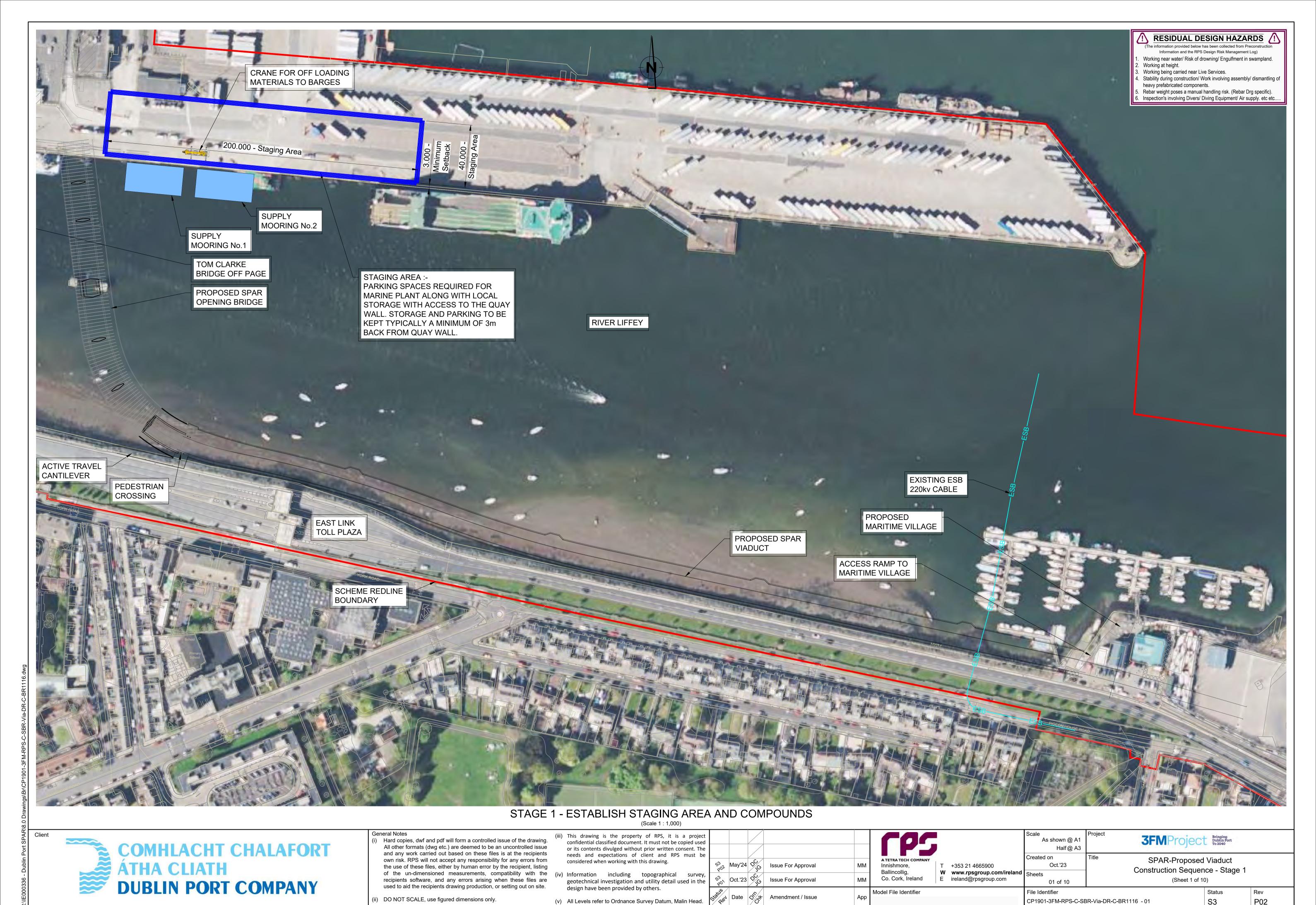
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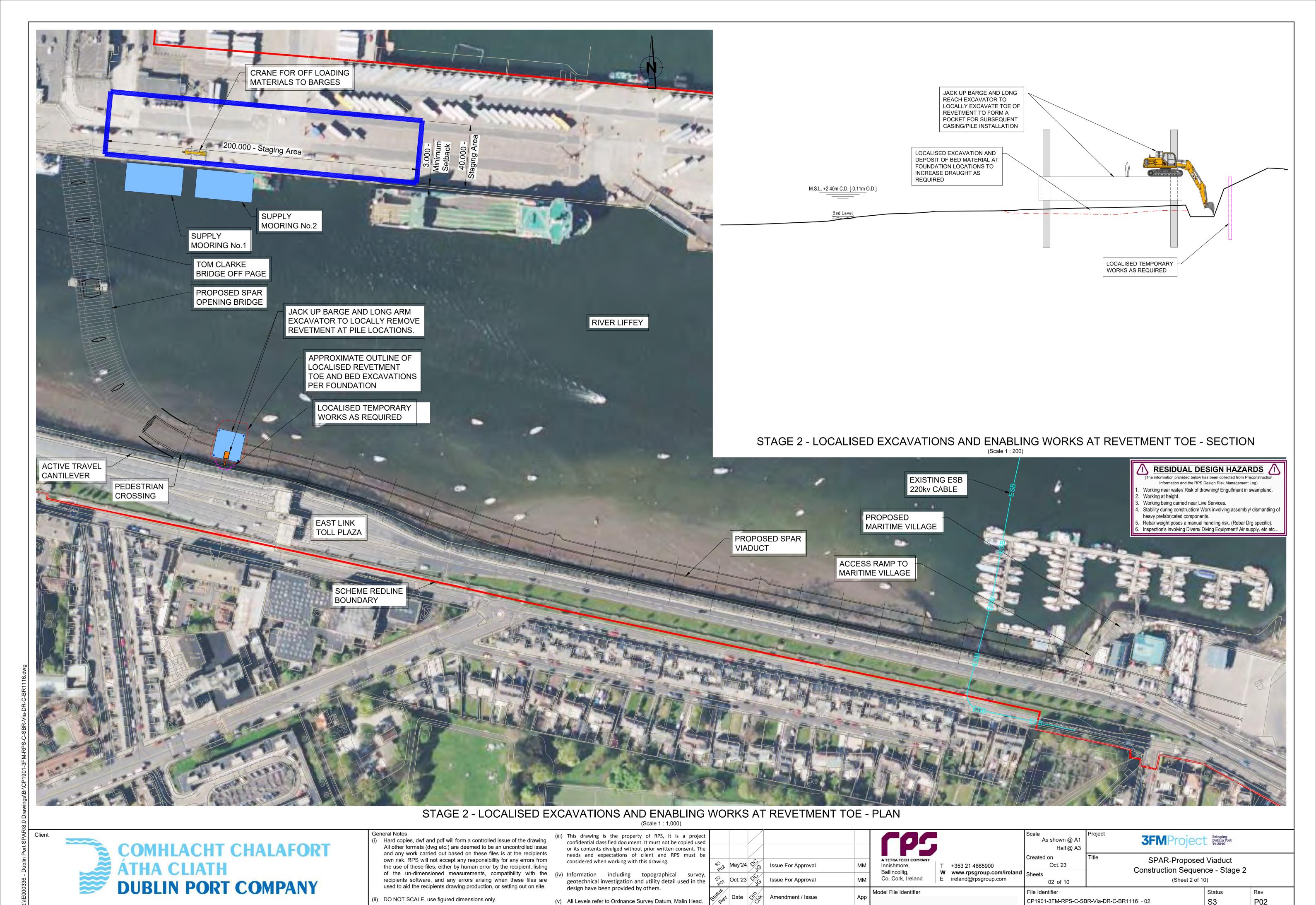
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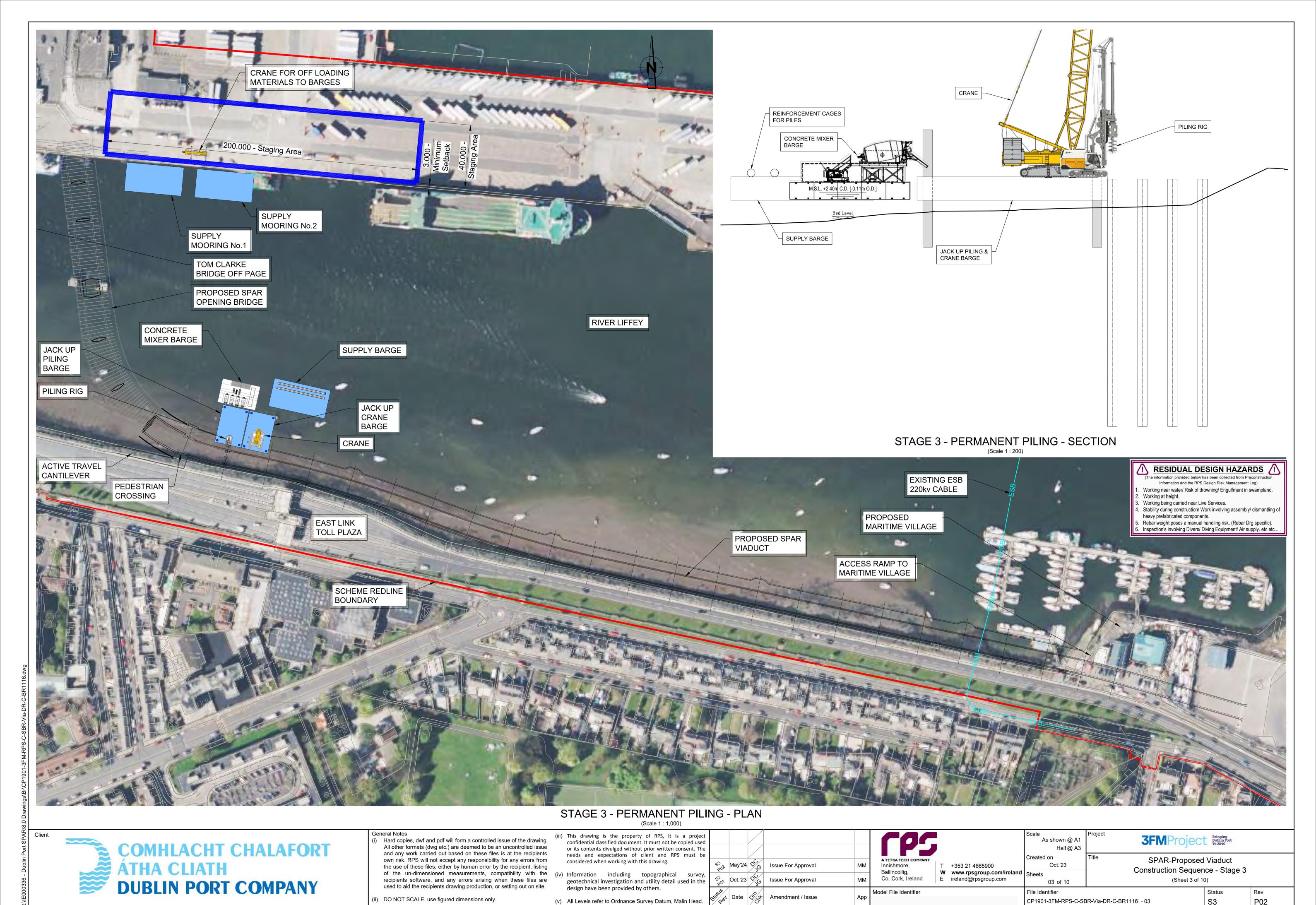
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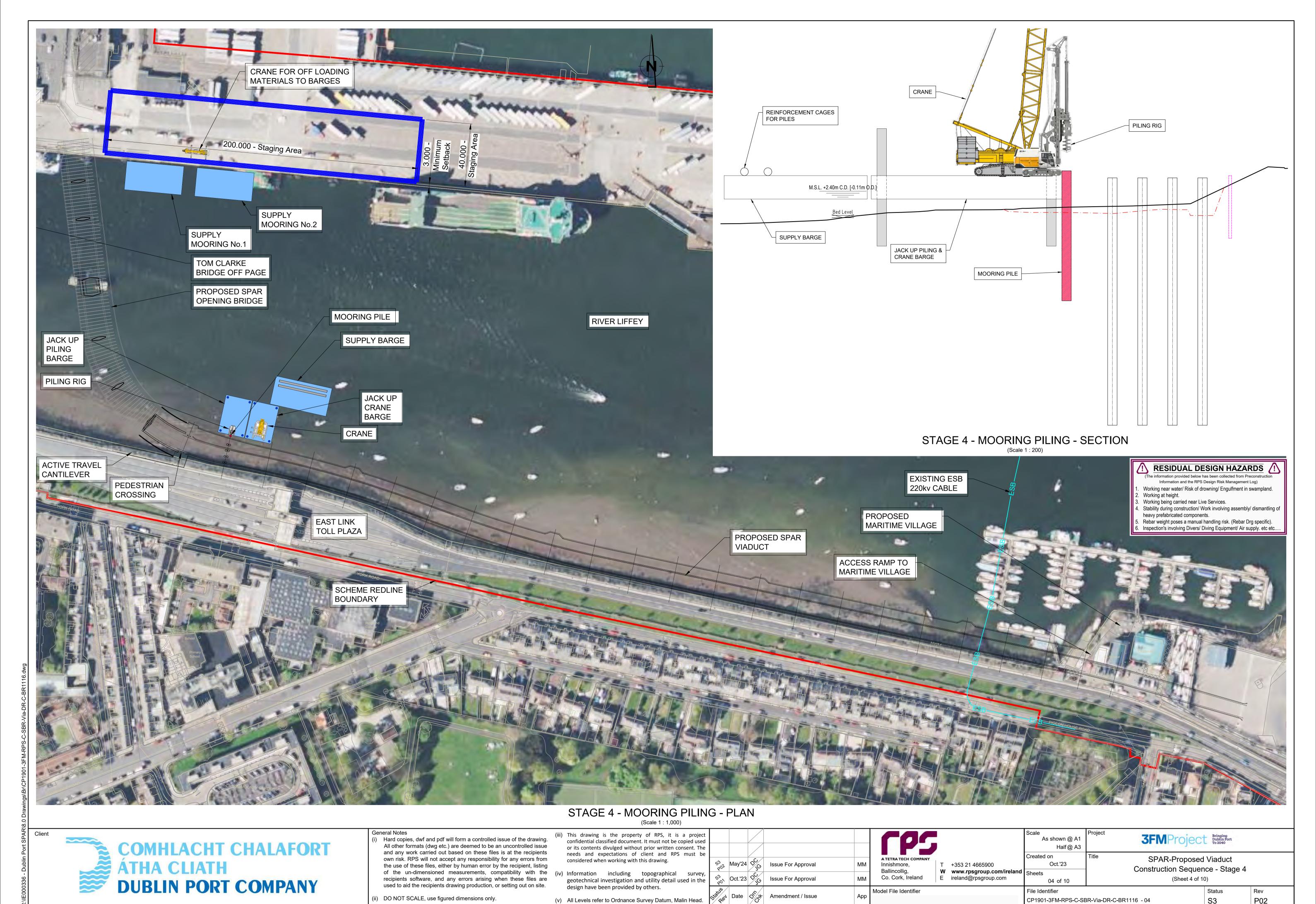
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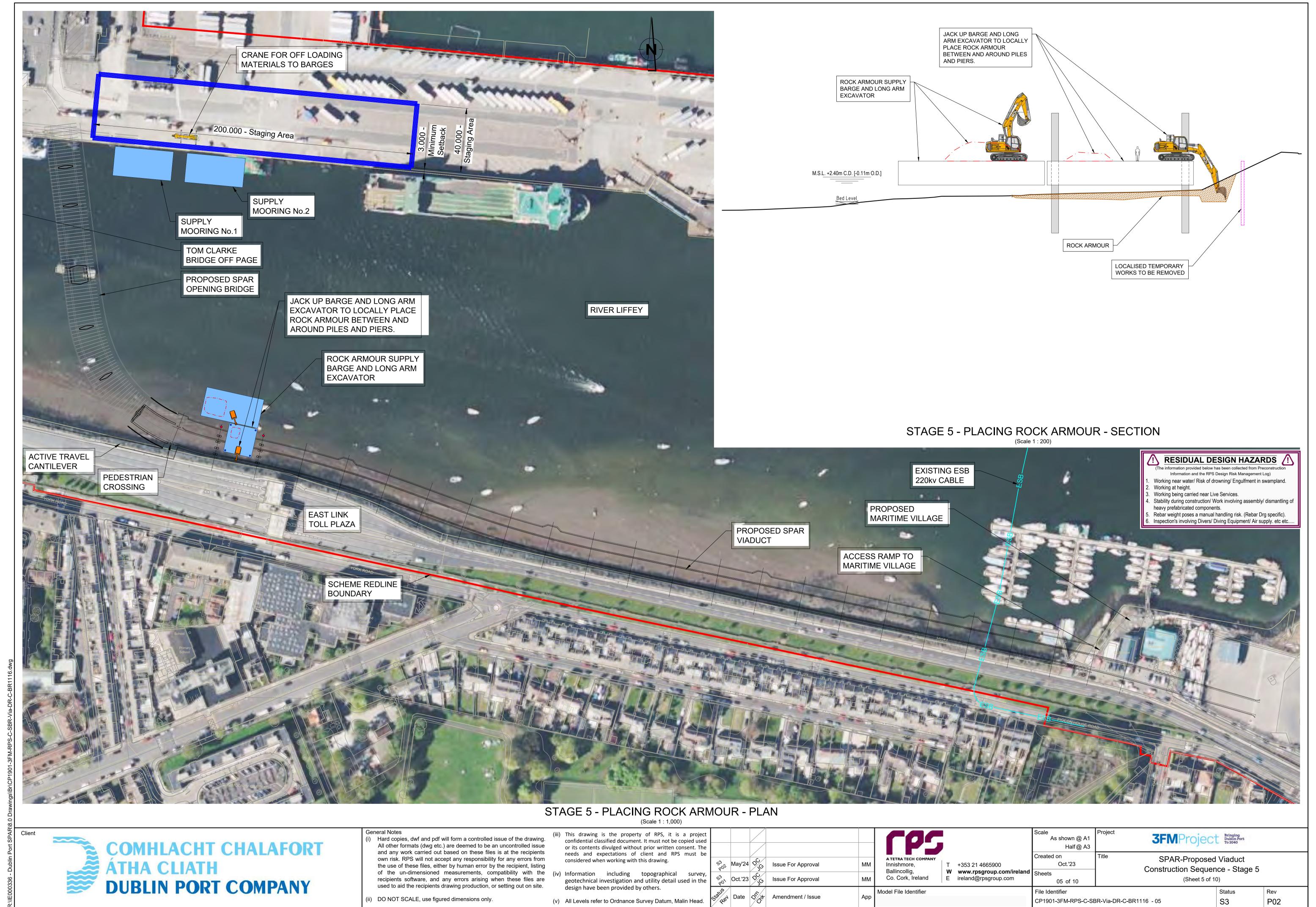
# Appendix C Construction sequence drawings

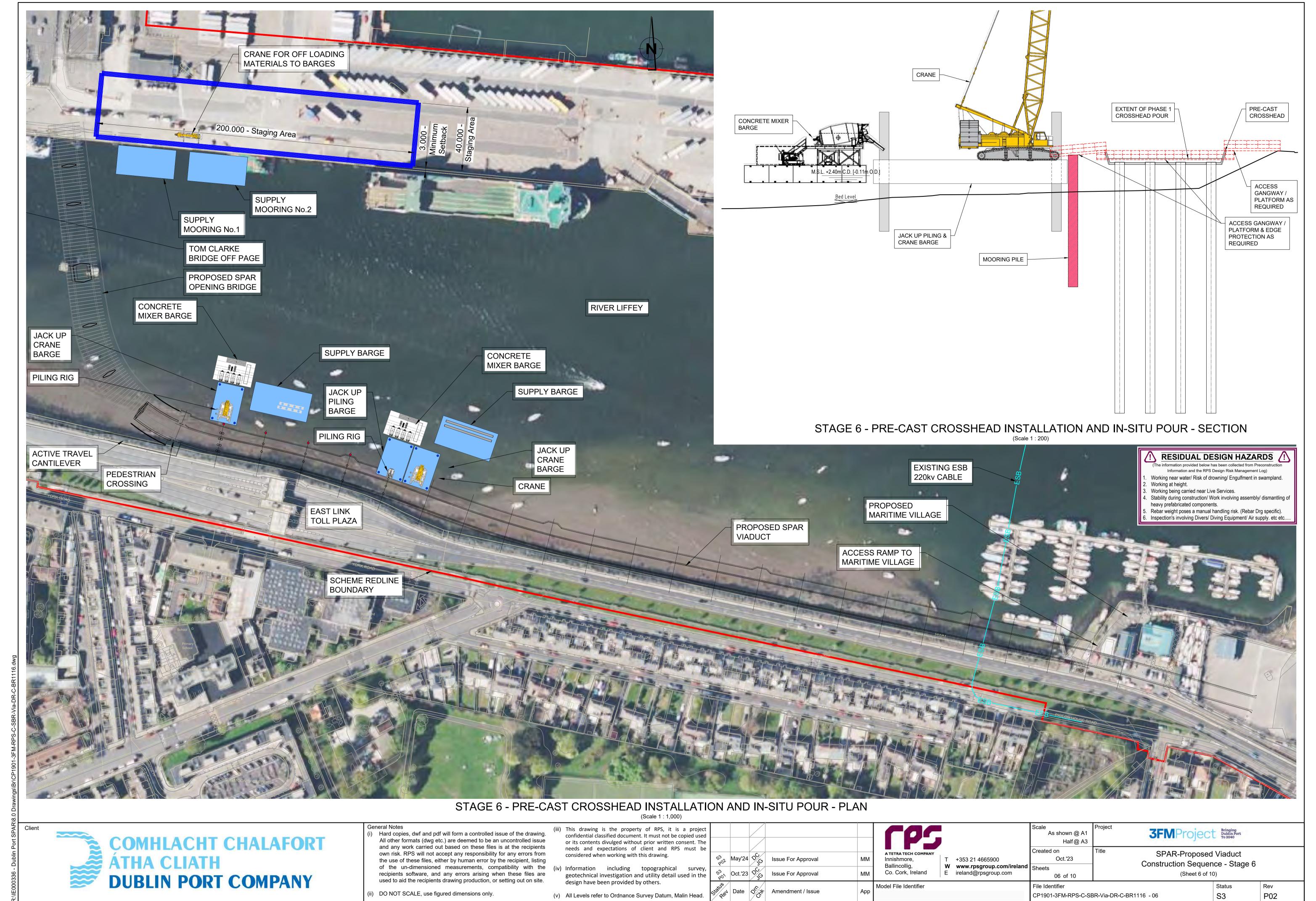


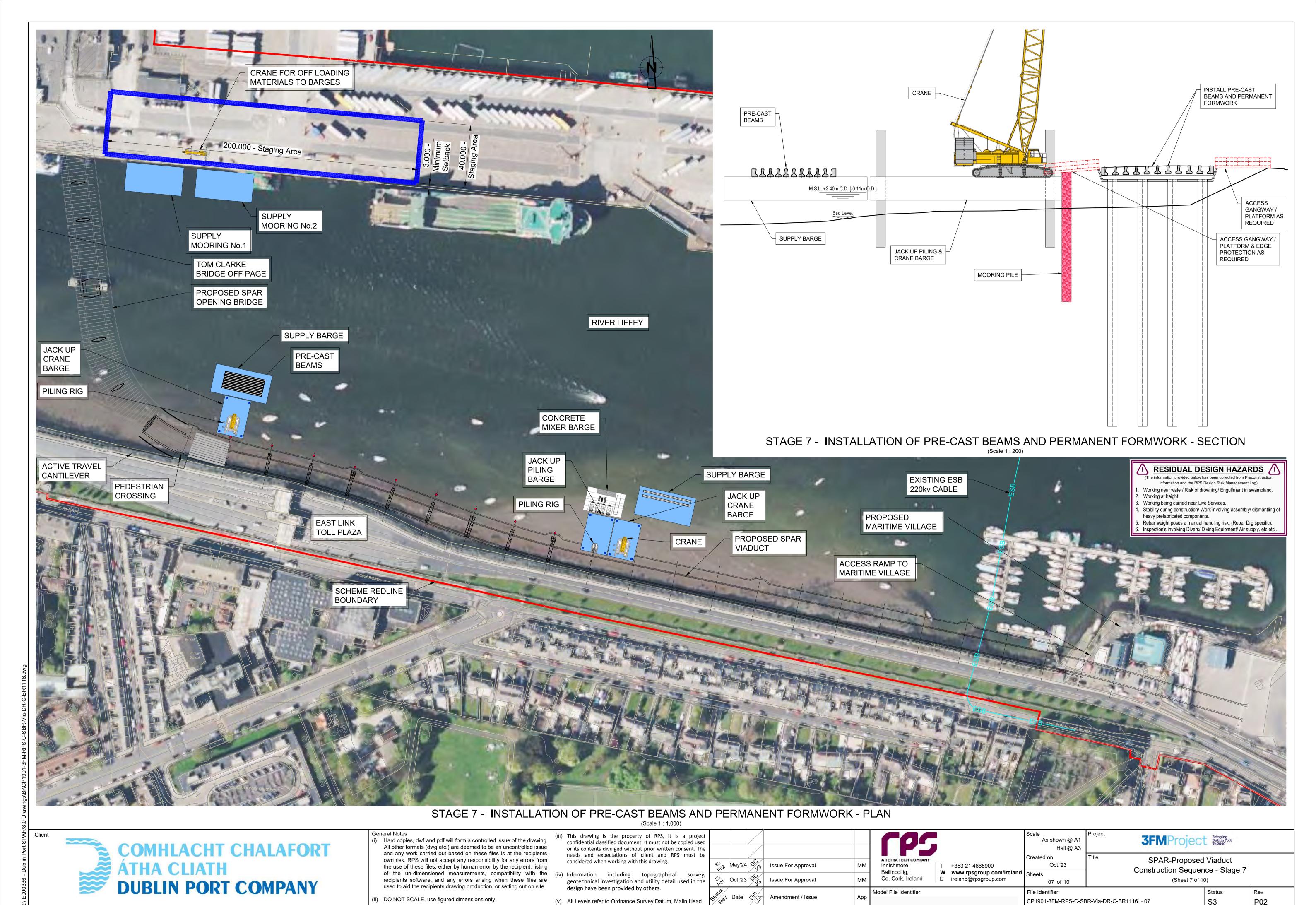


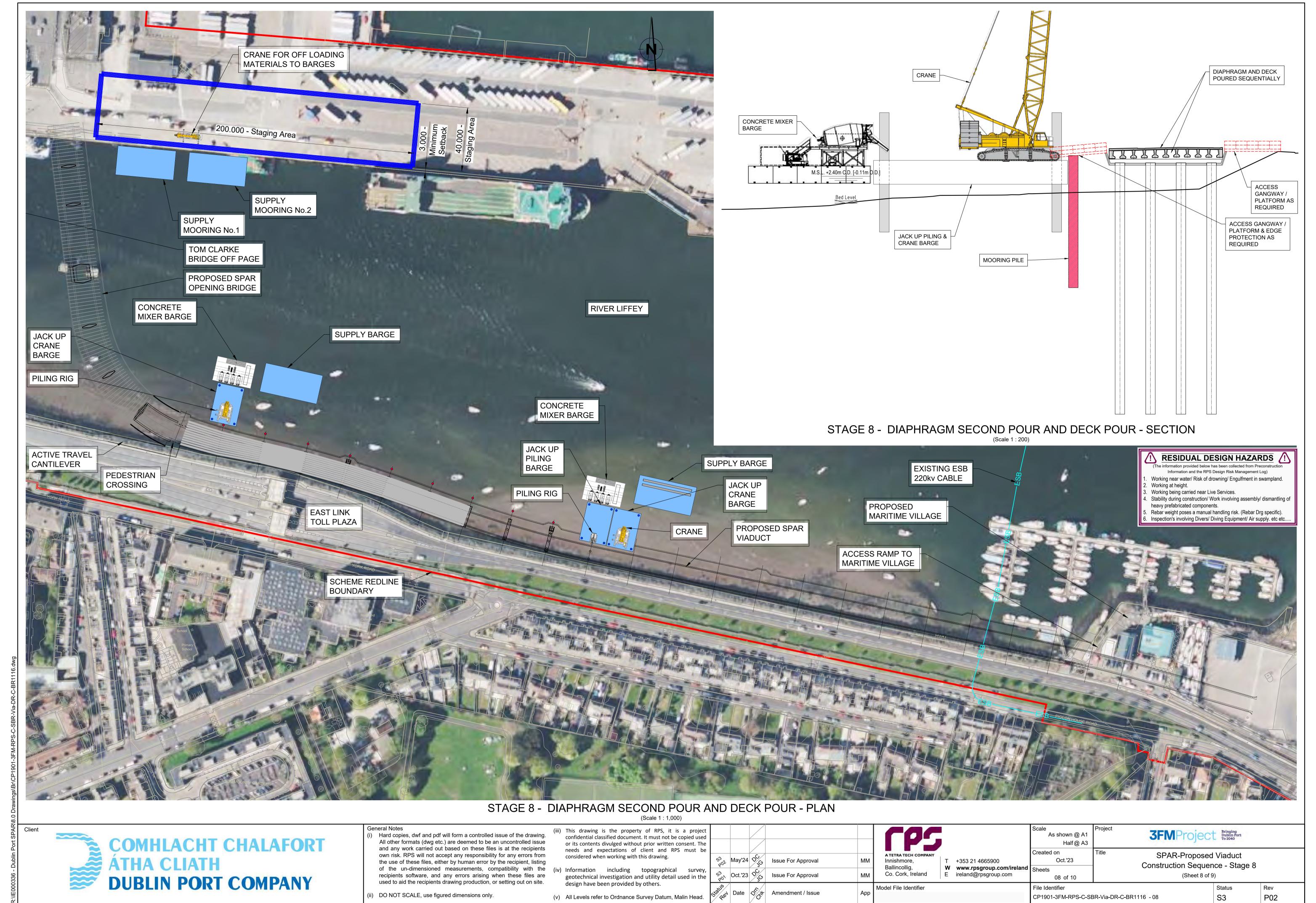


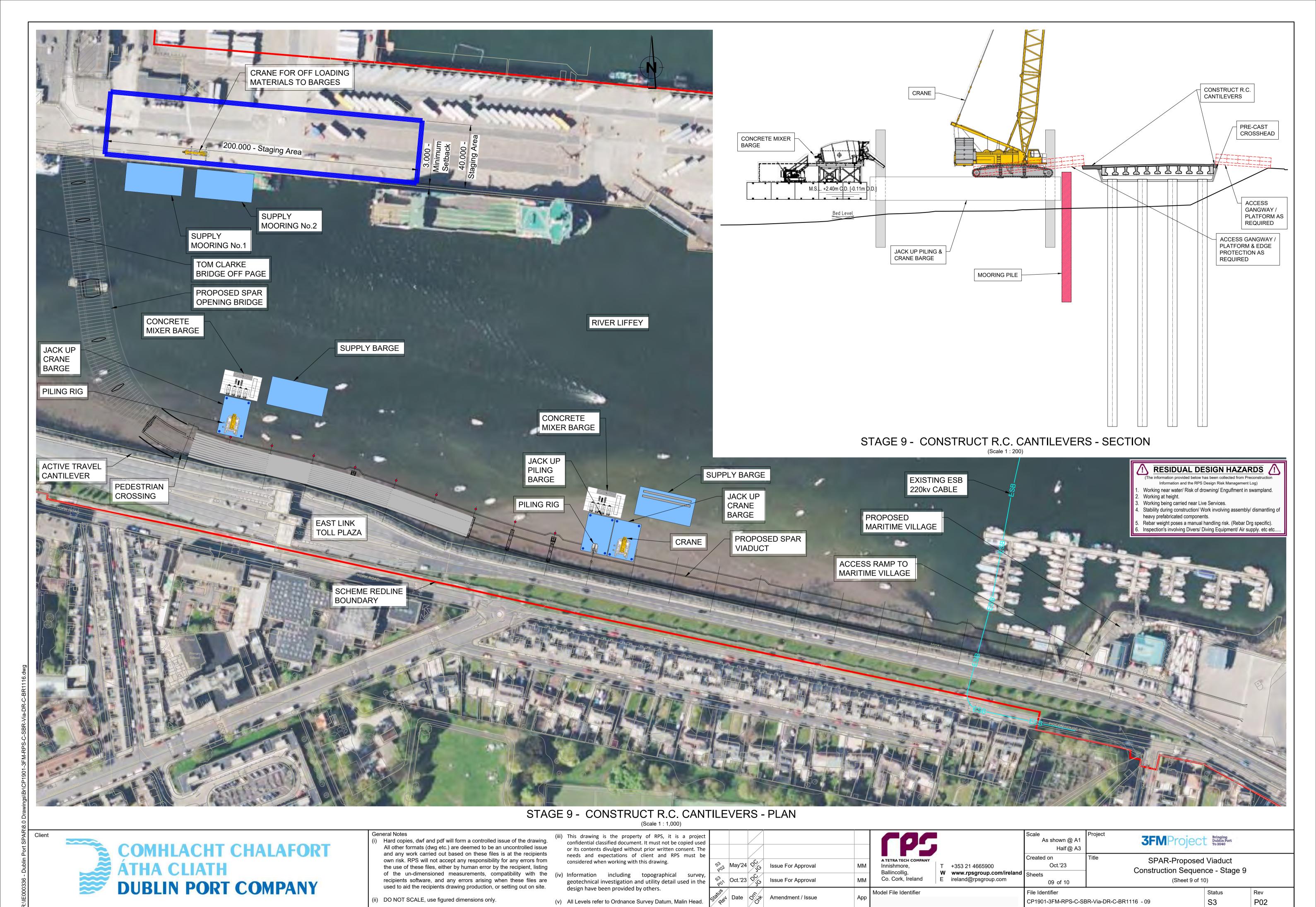


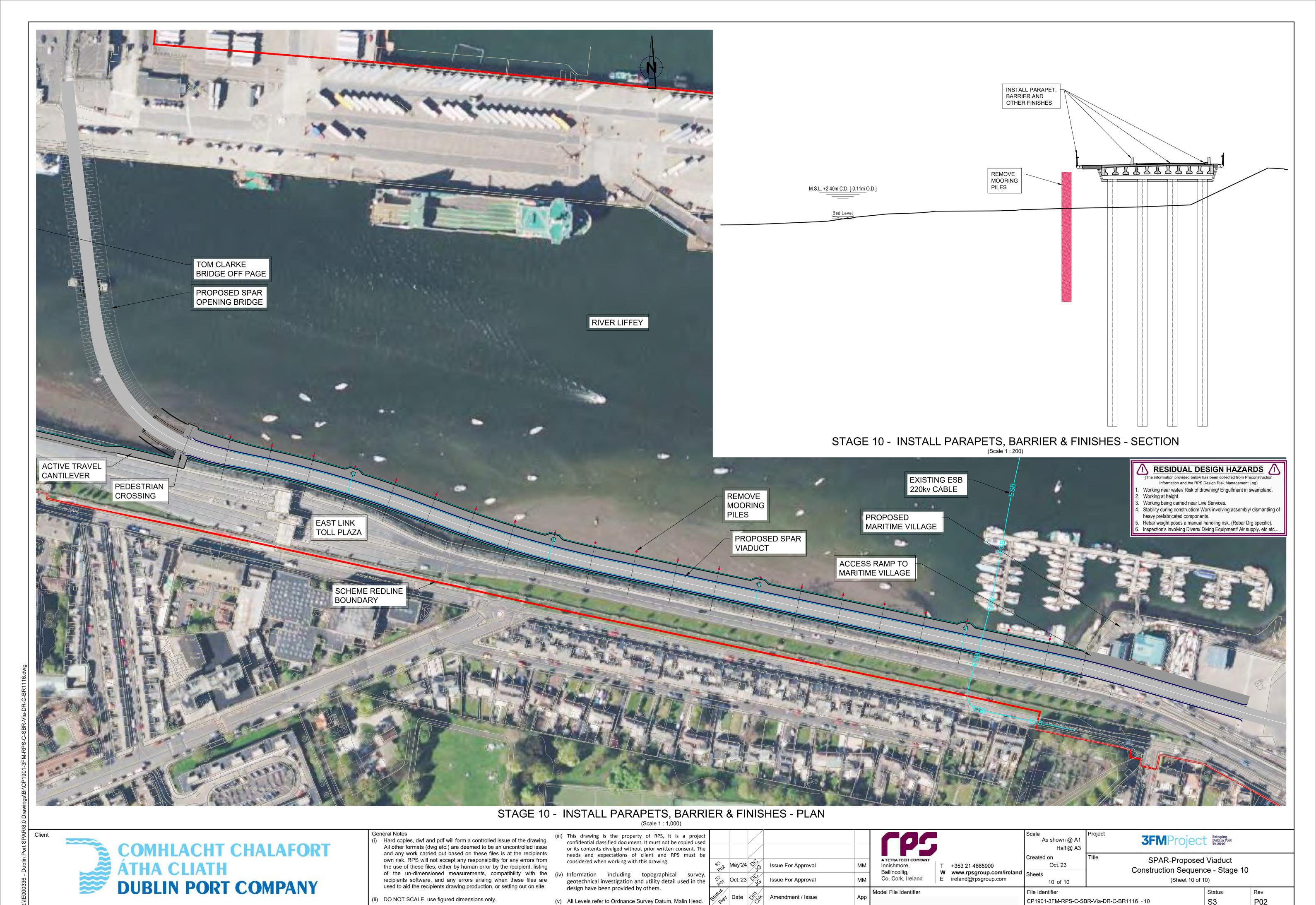




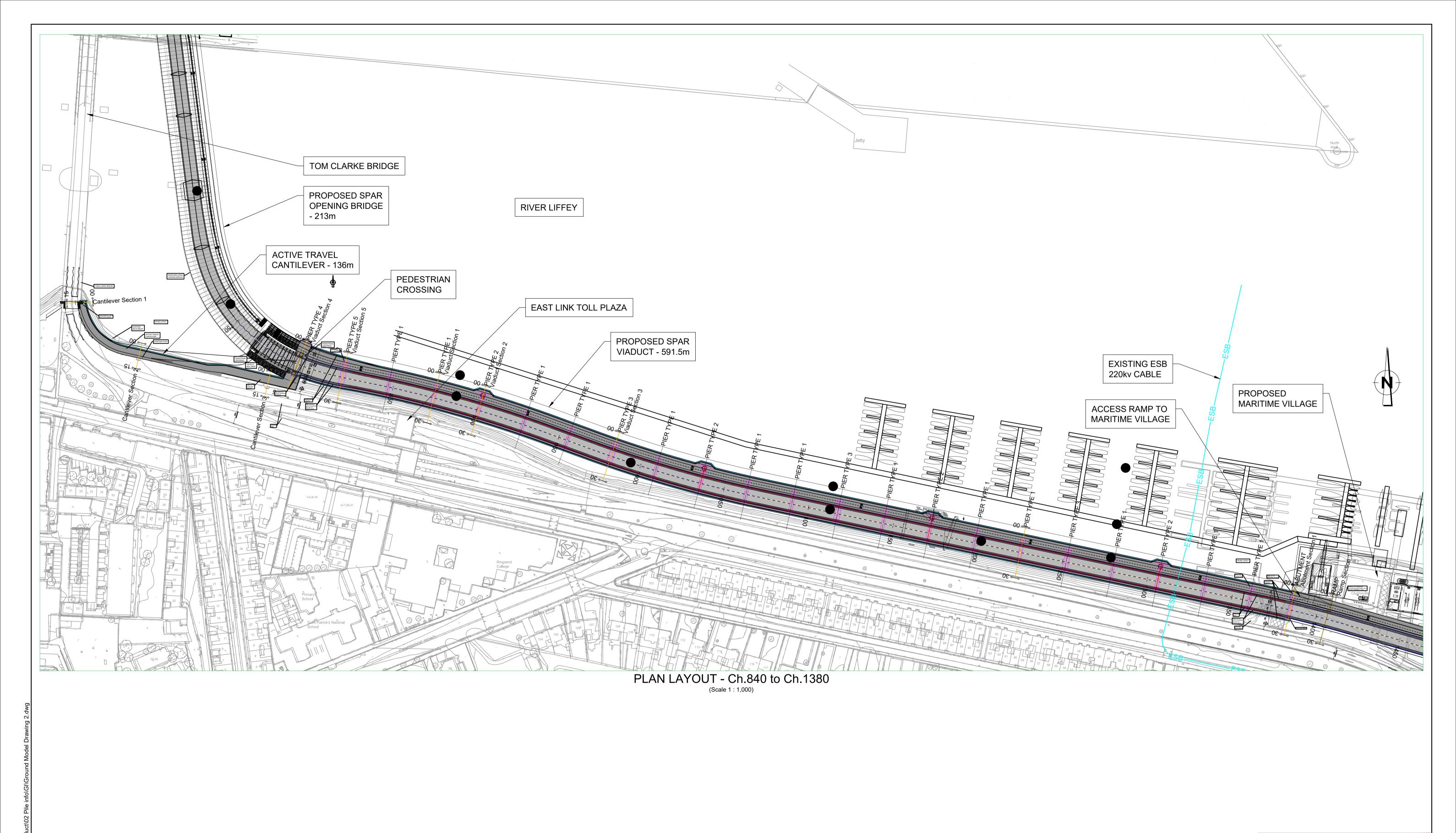








# Appendix D Geotechnical Information





- Information and the RPS Design Risk Management Log)
- Working near water/ Risk of drowning/ Engulfment in swampland.
- . Working at height.
- Working being carried near Live Services.
- 4. Stability during construction/ Work involving assembly/ dismantling of
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**3FM**Project Bringing Dublin Port To 2040 Half@ A3 SPAR-Proposed Viaduct Feb.'23 (Sheet 1 of 1) 01 of 7

As shown @ A1

Ground Model Drawing 2

PLAN LAYOUT

S3

P01.04

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•		_	rdinates (m)	F211	210				Ground Elevation (m Datum)	Sheet 1	of 6			
		Hole	Туре	Cabl	e Per	cussi	ion to	Rota	ary Coring	Status		Preli	minaı	у
Depth	Sampl	ing an	d In Situ Testing		ore Re		ery		Strata Details			Г	Grou	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
0.00 - 0.50 - 0.00 - 0.50 -	B D	1 2						-	Very soft black slightly sandy clayey SILT with rare decomposed wood fragments (<10mm) and moderate sulphurous odour. Sand is fine. [RECENT ESTUARINE DEPOSITS]					
- 0.50 - 0.95 0.50 - 0.95	D SPT	3	0/450 (S)					-		(1.00)		X		
- 1.00 - 1.50 1.00 - 1.50 -	B D	4 5						1-	Very soft to soft black slightly sandy silty CLAY with occasional wood fragments (<5mm x 30mm). Sand is fine. [RECENT ESTUARINE DEPOSITS]	1.00		X X X X X X X X X X X X X X X X X X X		
- - 1.50 - 2.50 1.50 - 2.50	B D	7 8						-	Firm black slightly sandy slightly gravelly silty	1.50		×_×_ ×		
1.50 - 2.50 - -	PS	6						-	CLAY with some shell fragments (<20mm) and rare brown silt burrows (<5mm x 30mm) and moderate hydrocarbon odour. Sand is fine. Gravel is angular fine to coarse of limestone. [RECENT ESTUARINE DEPOSITS]			× × × ×		
-								2		(1.00)		X X X X X X X X X X X X X X X X X X X		
- 2.50 - 2.95 2.50 - 2.95	D SPT	9	0/450 (S)					-	Very soft to soft black slightly sandy slightly gravelly silty CLAY with occasional rootlets (<1mm x 20mm) shell fragments (<50mm) of scallop and bivalves. Sand is fine. Gravel is angular fine to coarse gravel of limestone. [RECENT ESTUARINE DEPOSITS]	2.50		X X X X X		
2.95 - 3.00 - 3.00 - 3.50 3.00 - 3.50	D B D	10 11 12						3				× × × × × × × × × × × × × × × × × × ×		
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5.00 - 5.50 Notes	D	16							Continued flext page					

08/11/2022

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL Combined CP+RC FI.hbt/Config Fugro Rev5/23/12/2019/TS+AW

		Cont	ract Name	Dubl	in Po	rt MP	<sup>2</sup> _3F	-M		Locatio	n ID			
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•			dinates (m)						Ground Elevation (m Datum)	Sheet 2	of 6			
		Hole	Туре	Cabl	e Pei	cuss	ion to	Rota	ary Coring	Status		Preli	minar	у
Depth	Sampl	ing an	d In Situ Testing	Co	re R	ecove	ery		Strata Details				Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
- - - - 5.50 - 6.00	UT	17	22/450 mm					-	gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular and subrounded fine to coarse of limestone. [ESTUARINE DEPOSITS]	(1.00)		X X X X X X X X X X X X X X X X X X X		
- 6.00 - 6.20 - 6.20 - 6.20 - 6.50	B D B	18 19 20						6	Grey clayey very gravelly SAND. Sand is fine to coarse. Gravel is angular to rounded fine to coarse of limestone, quartzite and psammite. [ESTUARINE DEPOSITS]  Firm to stiff light brown slightly sandy slightly gravelly silty CLAY with occasional lenses (<150mm) of silty fine to coarse sand. Sand to	- 5.90 (0.30) - 6.20		× × × × × × × × × × × × × × × × × × ×		
- 6.50 6.50 - 6.95 - 6.50 - 6.95	D D SPT	21 22	N = 23 (S)					-	coarse. Gravel is angular to subrounded fine and medium of limestone and psammite. [ESTUARINE DEPOSITS]			X X X X X X X X X X X X X X X X X X X		
- - 7.00 - 7.50 - -	D	23						7 —		(1.30)		X_X_X_X_X_X_X_X_X_X_X_X_X_X_X_X_X_X_X_		
- 7.50 - 7.95 7.50 - 8.00 7.50 - 7.95	D B SPT	24 25	N = 12 (S)					-	Very soft to soft brown slightly sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is angular fine of limestone. [ESTUARINE DEPOSITS]	7.50		× × × × × × × × × × × × × × × × × × ×		
- - 8.00 - 8.50 - -	В	26						8 —	Stiff dark brown slightly sandy gravelly CLAY. Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of limestone, psammite and quartzite. [PORT CLAY]	8.00		• • • • • • • • • • • • • • • • • • •		
- 8.50 8.50 - 8.95 8.50 - 8.95 -	D D SPT	27 28	N = 27 (S)					- - - - 9 —						
- 9.20 - 9.50 9.50 - 9.95 9.50 - 9.95	B D SPT	29 30	N = 27 (S)					- - - -		(3.00)		* · · · · · · · · · · · · · · · · · · ·		
- - -10.00 - 10.50 Notes	В	31						- - -	Continued next page			* * * * * * * * * * * * * * * * * * *		

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-Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL Combined CP+RC FI.hbt/Config Fugro Rev5/23/12/2019/TS+AW

		Cont	tract Name	Dubl	in Po	rt MP	2_3F	=M		Location	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	ny		3FN	VI	BH-	S-	.01
			o Reference	F211						<b> </b> • • • • • • • • • • • • • • • • • • •	··—			•
•			rdinates (m)						Ground Elevation (m Datum)	Sheet 3	of 6			
			Туре	Cabl	e Pei	rcussi	ion to	Rota	ary Coring	Status		Preli	minar	у
Depth	Samp	ling an	nd In Situ Testing	Co	re R	ecove	ery		Strata Details				Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datun	Legend	Water Strike	Backfill / Installation
Depth (m)  - 10.50 - 10.95 - 10.50 - 10.95 - 10.50 - 10.95 11.00 - 11.50 11.50 - 11.94 - 11.50 - 11.94 12.00 - 12.50 12.50 - 12.90 13.50 - 13.95 - 13.50 - 13.95	Type  D SPT  B  D SPT  B  D SPT	No. 32 33 34 35 36 37		TCR (%)	SCR (%)	RQD (%)		Depth (m)	Very stiff dark grey sandy gravelly CLAY with low cobble content. Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of limestone, psammite, quartzite. Cobbles (<140mm x 80mm) are angular of dark grey limestone. [PORT CLAY]	(Thickness)	Level (m Datun	Legend  Legend		Backfill / Installation
-14.00 - 14.50 - - - - - - - - - - - - - - - - - - -	D SPT	39 40	50/285 mm (S)											
—15.00 - 15.50	В	41						_	Continued next page					

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-Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL Combined CP+RC FI.hbt/Config Fugro Rev5/23/12/2019/TS+AW

		Conf	tract Name	Dubl	lin Po	rt MP	<sup>2</sup> _3F	-M			ח וט		
-fugi	RO					ort Cor	mpar	ny		3FI	<b>V</b> _[	BH-	<b>-S-0</b>
			ro Reference	F211	210				One and Elevation (as Datum)				
			ordinates (m) e Type	Cab	le Pe	rcuss	ion to		Ground Elevation (m Datum) ary Coring	Sheet 4 Status	of 6	Preli	iminary
	Camp		nd In Situ Testing			lecove		) No.	Strata Details	Status		1 10	Groundwa
Depth (m)		Γ	Test		ore Re		_	Contr	Ī	Depth			Water D
	Туре	No.	Results	(%)			(-)	Depth (m)	Strata Descriptions	(Thickness) (m)	(m Datum	Legend	Water Back Strike Instal
15.50 - 15.87 15.50 - 15.88	D SPT	42	50/225 mm (S)					-					
16.00 - 16.40	В	43						16 —					
16.40 - 16.64 16.40 - 17.10	SPT		50/85 mm (S)	71	0	0		-	Very dense dark grey and greenish grey slightly sandy clayey GRAVEL with low cobble content and rare seams (<70mm) of dark grey sandy gravelly clay. Gravel is angular to subrounded fine to coarse of limestone psammite. Cobbles (<140mm x 90mm) are angular to subrounded				
								17 —	of strong dark grey limestone. [GLACIAL DEPOSITS]  16.40m to 16.60m; assumed zone of core loss.				
17.10 - 17.80				100	0	0		-		(2.20)			
17.80 - 17.84	SPT		50/0 mm (C)	87	0	0		18 —	17.80m to 17.90m; assumed zone of core loss.				
18.60 - 18.69	С	44						- -	Medium strong and strong dark grey and grey	18.60			
18.60 - 18.85							4	19 —	LIMESTONE with rare inclined (70-90°) calcite veins (<30mm). Fresh locally distinctly weathered and destructured. Discontinuities; bedding plane fractures, inclined (0-10°), closely spaced, planar, smooth, very tight,				
18.95 - 19.13 18.60 - 19.50 19.13 - 19.25 19.35 - 19.40				89	33	14	-		clean. Set#2, joints, inclined (80-90°), closely spaced, planar, smooth, very tight, clean.  [LIMESTONE BEDROCK]  19.85m to 18.95m; extremely weak. Destructured with very weak angular fine to coarse gravel-sized lithorelicts.				-
19.50 - 19.74							8	-	19.13m to 19.25m; non-intact. Recovered as angular and subangular fine to coarse gravel-sized fragments.     19.25m to 19.35m; very weak. Destructured with weak angular and subangular fine to coarse gravel-sized lithorelicts.				
19.74 - 20.01							-	.	graversize inforences  19.35m to 19.40m; non-intact. Recovered as angular and subangular fine and coarse gravel- sized fragments.  19.40m to 19.50m; assumed zone of core loss.  Continued next page				

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		Cont	ract Name	Dubl	in Po	rt MP	2_3F	М		Location	n ID
-fug	RO	Clien	t	Dubl	in Po	rt Co	mpar	ny		3FN	И_ВН-S-01
		Fugr	o Reference	F211	210						
Y		-	dinates (m)						` /	Sheet 5	
		Hole	Туре	Cabl	e Pei	rcuss	ion to	Rota	ary Coring	Status	Preliminary
Depth	Sampli	ing an	d In Situ Testing			ecove	ery		Strata Details		Groundwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum) Legend Water Strike Installation
20.03 - 20.20	С	45						_	19.74m to 20.01m; non-intact. Recovered as angular fine to coarse gravel and cobble-sized		
19.50 - 21.00 20.01 - 20.59				90	60	46	9	-	fragments.		
- - - 20.73 - 20.85							8	-	20.59m to 20.73m; distinctly weathered with extremely closely spaced bedding plane fractured to destructured with angular fine and medium gravel-sized lithorelicts.  20.85m to 21.00m; assumed zone of core loss.	(4.30)	
- 21.00 - 21.15 21.10 - 21.26 21.15 - 21.20	С	46					13	21 —	21.15m to 21.20m; non-intact. Recovered as angular fine and medium gravel-sized fragments.		
21.20 - 21.43							4	-	g grave oizou nagmonia.		
-								-	21.43m to 21.69m; distinctly weathered with extremely closely spaced bedding plane fractures.		
21.00 - 22.50				100	70	31		22 —			
- - 21.69 - 22.90							8	-	22.12m to 22.23m; inclined (85°) calcite vein (<20mm).  22.32m to 22.50m; inclined (70°) calcite vein		
- - 22.65 - 22.70	D	47						-	(<35mm).  22.50m to 22.72m; inclined (70°) calcite vein (<20mm).		
- - 22.90 - 22.97							-	-	Medium strong to strong locally very weak dark	22.90	
22.50 - 24.00 23.04 - 23.55				100	33	0	18	23 —	grey to black LIMESTONE. Fresh to slightly weathered locally destructured. Predominately non-intact, recovered as angular coarse gravel and cobble-sized fragments. Discontinuities; Set#1, bedding plane fractures, inclined (0-10°), very closely spaced, planar, smooth, clean. Set#2, joints, inclined (60-80°), planar and undulating, smooth, clean. [LIMESTONE BEDROCKI		
- - 23.70 - 23.96 -	В	48						-	22.90m to 22.97; non-intact. Recovered as angular coarse gravel-sized fragments. 22.97m to 23.04m; very weak. Destructured with angular fine and medium gravel-sized lithorelicts. 23.04m to 24.00m; partially non-intact recovered	(1.43)	
23.55 - 24.33							-	24 —	as angular coarse gravel-sized fragments. Bedding plane fractures present. 24.00m to 24.33m; non-intact, Recovered as angular fine and medium gravel-sized fragments.		
24.55 - 24.88 24.00 - 25.50	С	49		100	70	47		- - - - -	Strong dark grey and black locally grey LIMESTONE with rare inclined (70-90°) calcite veins (<3mm). Fresh locally destructured. Discontinuities; Set#1, bedding plane fractures, inclined (0-10°), extremely closely to medium spaced, planar and undulating, smooth, tight to very tight, clean. Set#2, joints, inclined (80-90°), planar and undulating, smooth and rough, very tight to tight, clean and with calcite coating.	24.33	
									Continued next page		
Notes - Abbreviations	and res	ults dat	ta defined on 'Note	s on E	Explor	atory I	Positio	on Red	cords'		

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		Cont	ract Name	Dubl	in Po	rt MP	2_3F	М		Locatio	n ID		
-fug	RO	Clien	t	Dubl	in Po	rt Co	mpar	ıy		3FI	<b>M</b>	BH-	S-01
		Fugr	o Reference	F211	210						_		
· ·			dinates (m)						Ground Elevation (m Datum)	Sheet 6	of 6		
		Hole	Туре	Cabl	e Per	cussi	on to	Rota	ary Coring	Status		Preli	minary
Depth	Sampli	ing and	d In Situ Testing	Co	re Re	ecove	ery		Strata Details	_			Groundwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum	Legend	Water Backfill / Installation
24.33 - 25.70							16		[LIMESTONE BEDROCK]				
24.33 - 25.70  - 25.50 - 25.67  - 25.70 - 25.81	C	50	Results	100	70	14 44		(m)		(4.17)	, (in Datum		Strike installation
								-					
_								_					
N1-4											1		
Notes - Abbreviations	and res	ults dat	a defined on 'Notes	s on E	xplora	atory F	Positio	on Re	cords'				

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Print Date

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			1			$\overline{}$													ocati	on ID				_
			Coi	ntract Na	ame	[	Dubli	n Por	t MP	2_3FN	Л							-	ocau	on ID				
-fu	G	RO	Clie	ent	_		Dubli	n Por	t Cor	mpany	/				_				3F	M	Bł	┨.	<b>-S-0</b>	1
			<del>-</del>	gro Refe			-211	210				Τ_								_				
			-	ordinates le Type	s (m)		ahla ahla	Dar	oueei	on an	d Rotar		d Eleva	ition (m	n Dat	um)		_	heet tatus	1 of 1		Prali	iminary	$\dashv$
			По	le Type			Jabi	Fei			Penetr			sults					ldius			1611	Milliary	$\dashv$
Test Dept	h (m)	Test Ty	/ре	Self Weig Penetration	ght (mm)	Test Re	esult						Total Per (mi	netration	Ham	mer Serial lumber	Ener	gy Rat	io (%)	Casing	Depth (n	n) \	Water Depth	ı (m)
0.50 2.50		S S		0		N=0 (0 N=0 (0							45 45	0	AF	R3214 R3214		54 54			0.50 2.50	T	0.00	
4.50	)	S		0		N=0 (0	,0/0,0	0,0,0					45	50	AF	R3214		54		4	4.50		0.00	
6.50 7.50	)	S S		0		N=23 ( N=12 (	5,4/4	,3,2,3	)				45 45	60	AF	R3214 R3214		54 54		7	6.50 7.50		0.00	
8.50 9.50	)	S S		0 0		N=27 ( N=27 (	4,7/7	,5,7,8	)				45 45	50	AF	R3214 R3214		54 54		9	3.50 9.50		0.00 0.00	
10.50 11.50		S S		0		N=36 ( N=50 (				m)			45 44			R3214 R3214		54 54			0.50 1.50		0.00	
12.50 13.50	)	S		0		N=50 ( N=42 (	8,9/5	0 for 2	245mı				39 45	5	AF	R3214 R3214		54 54		1	2.50 3.50		0.00	
14.50	)	S		0		N=50 ( N=50 (	4,8/5	0 for 2	28Śmi				43	35	AF	R3214		54		1	4.50 5.50		0.00	
15.50 16.40	)	S		0		N=50 (	18,7	50 for	85mi	m) <sup>´</sup>			37 23	5	AF	R3214 R3214		54 54		1	6.40		0.00	
17.80	0	С		0		N=50 (	(25 to	r 35m	m/50	for 0m	m)		3	5	Al	R3214		54		1	7.80		0.00	
Test Depth	In Test	T Und	disturbe	Test Resu	Resid	lual Undrai	ned	_		Situ Ha Depth (r	and Per		ed Undraine	ed Shear St	rength		e Head st Dep			ng by I			ion Detect	or
(m)	iest	sype st	near Str	rength (kPa)	Shear	Strength (I	kPa)		iesi D	repui (i	11)		(kPa	)		16	si Del	701 (11	')	+	FIDIN	csu	іт (ррііі)	_
Notes					c			_			D :::	-												
- Abbrev	lation	s and r	esuit	ts data d	etine	a on 'r	votes	on E	zxpio	ratory	Positio	n Reco	oras'											

Template: FGSL/HBSI/FGSL SPT Summary.hbt/Config Fugro Rev5/18/02/2019/TS

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			Со	ntrac	ct Nam	ie [	Dublir	Port MP	2_3FM									Loc	ation	ID		
-fu	IGR	{•	Clie	ent		1	Dublin	Port Co	mpany									3	FN	l B	H-\$	S-02
					Refere		-2112	10												_		
,			_		nates (		<u> </u>	<u> </u>					Elevation	(m D	atuı	m)		_	et 1 c	of 1	D ::	
			Ho	le Ty	/ре	(	Jable	Percussi	ion to R									Stat	ius		Prelim	inary
Depth From	Depth To (m)	Hole	Type	Date	From	Date To	-	quipment	Core Ba		uipm Con	ent e Bit	Drilling Crew	Logge	d Bv	Remar	ks					
0.00	14.56	C		l	1/2022	03/11/2022	С	omacchio	0016 D		501		CR, AB, JW,	SI, F		· vorrial						
14.40	28.60	P	С	03/4-	1/2022	05/11/2022	1	S1200, Pilcon Wayfarer cchio MCS120	00				SB CR, AB	SI, F	PG							
14.4U	20.00		~	03/1	1/2022	00/11/2022	Comac	onio ivioo i2l	,,,				OR, AB	31, 1	J							
					Progre									otary I							Core De	
Date (dd/mm/yyyy 03/11/2022		n)	Hole De (m) 0.00	)	(m) 0.00	h Water Depth (m)	Weath	er		From 14.	n (m)	(m) 15.10	Flush Ty W	ре		h Return (%) 100	Flush Colo Grey	ui (t	un Time hh:mm) 00:06	Depth From (m) 14.40	Depth To (m) 15.10	Diameter (mm)
03/11/2022 03/11/2022 03/11/2022	2 18:4	0	10.8	38	10.50 10.50		l' all			15. 15.	.10	15.10 15.85 16.60	W W		'	100 100 100	Grey Grey Grey		00:06 00:04 00:05	15.10 15.85	15.10 15.85 16.60	102 102 102
03/11/2022 04/11/2022	2 23:1	8	14.5 14.4	66 10	14.50 14.40					16. 17.	.60	17.60 18.10	W W			100 100 100	Grey Grey		00:05 00:05	16.60 17.60	17.60 18.10	102 102
05/11/2022			28.6	80	14.40					18. 18.	.10 .85	18.85 19.60	W W			100 100	Grey Grey		00:05 00:06	18.10 18.85	18.85 19.60	102 102
										19. 21.	.10	21.10	W W		'	100 100	Black Black		00:20 00:16	19.60 21.10	21.10 22.60	102 102
										22. 24.	.10	24.10 25.60	W W		'	100 100	Black Black		00:18 00:22	22.60 24.10 25.60	24.10 25.60	102 102 103
										25. 27.	.10	27.10 28.60	W W			100 100	Black Black		00:20 00:20	25.60 27.10	27.10 28.60	102 102
				Holo	and C	lasing				$\left\{ \right.$												
Depth	To (m)	Hole		neter (n		Depth To (	m)	Casing Diar	meter (mm)	1												
10.	.88		20	00		10.20 14.40	-	20	00	1												
28.			14			14.40																
						w Progre		_		1												
Depth F			Depth 9.4	To (m)		Duration (hh: 01:18	mm)	Tool / R	Remark	-												
10. 14.	.20		10. 14.	.40		00:42 01:30																
	-					21.00																
		W	ater	Strik	(e			Water	Added	1												
Strike At (m)	Rise To (m)	Time	Elapseo	- 1	sing Depth	(m) Depth Se	aled (m)	Depth From (m)	Depth To (m)	1												
			-							1												
										<u> </u>							<u> </u>					
Groundwate	er observation				Rema	arks		Pa	rehole adv	anced	IIsina a	ahle no	rcussion with s				marks	teetin	n (SPT)	Client incl	ructed ret	ary switch at
2.24.14Wate				_ 0701									at 28.60m metr					i lestin	y (3P1).	Ciletti Inst	rucieu rota	ary switch at
		In	stalla	ation	1						Pipe								Bac	kfill		
Туре	Tip Depth . Distance (n	/ Res	sponse 2 Top (m	Zone F	Response Z Base (m	one Installation	n Date	ID To	op Depth (m)	Base	Depth (	m) Diar	meter (mm)	Туре	D	epth Fro		To (m)	) E	Backfill Mat	erial	Date
																0.00	28	3.60		Grout		05/11/2022
Notes										1												
	ations an	d res	ults c	data d	defined	in 'Explora	atory L	ocation R	ecords K	eysh	eets'											
						•	,			•												
L		_	_	_			_							_				_	_			
Checked By	'						E	levation Datu	m	D	ublin Po	ort Char	t Datum		G	Grid Coo	rdinate Syst	em	ITM			
Template: F	GSL/HBSI/F	GSL B	H Sum	nmary.h	nbt/Config	Fugro Rev5/	26/06/20	)19/TS+AW										Print I	Date		07/11/202	22

Location ID Contract Name Dublin Port MP2\_3FM 3FM\_BH-S-02 Client **Dublin Port Company** Fugro Reference F211210 Coordinates (m) Ground Elevation (m Datum) Sheet 1 of 6 Hole Type Cable Percussion to Rotary Coring Preliminary Status Sampling and In Situ Testing Core Recovery Strata Details Groundwater Depth (m) Test TCR SCR RQD Depth (m) Water Туре No. Strata Descriptions Legend Results (%) (%) (%) (-) Strike 0.00 - 0.50 0.00 - 0.50 B D Very soft black slightly sandy clayey SILT with strong sulphurous odour. Sand is fine. [RECENT ESTUARINE DEPOSITS] 0.50 - 0.95 0.50 - 0.95 D SPT 3 (1.00)0/450 (S) 1.00 - 1.50 1.00 - 1.50 B D 4 5 Very soft black slightly sandy silty CLAY with × rare plastic fragments (<0.1mm x 50mm x 100mm) and slight to moderate hydrocarbon odour. Sand is fine. [RECENT ESTUARINE DEPOSITS] × × × × 1.50 - 2.50 1.50 - 2.50 6 PS 2 250 - 295 D 8 2.50m to 3.00m; moderate hydrocarbon odour. 2.50 - 2.95 SPT 0/450 (S) (3.70)3.00 - 3.50 3.00 - 3.50 9 10 3 3.00m to 3.50m; rare plastic fragments (<0.1mm x 50mm x 100mm). | X | X | X 3.50 - 4.50 PS 11 × × × × 4 × D × 4.50 - 4.95 4.50 - 5.00 4.50 - 4.95 B SPT 13 N = 4 (S)4.70 Firm to stiff brown slightly sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to  $\times$ × (0.30)coarse of limestone and quartzite. 5.00 - 5.50 5.00 - 5.50 В 14 5.00 Continued next page

Print Date

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Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Cont	tract Name	Dubl	in Po	rt MF	P2_3F	-M		Locatio	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	าง		3FI	M F	ЗΗ.	S-	02
		-	o Reference	F211			•	,		<b>O</b>	··—·	<b>-</b>		<b>-</b>
•		Cooi	rdinates (m)						Ground Elevation (m Datum)	Sheet 2	of 6			
		Hole	Туре	Cabl	e Pei	cuss	ion to	Rota	ary Coring	Status		Preli	minar	ry
Depth	Sampl	ling an	d In Situ Testing	Co	ore R	ecove	ery		Strata Details				Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
-									[ESTUARINE DEPOSITS]   Firm to stiff grey silty CLAY. [ESTUARINE DEPOSITS]	(0.50)		X X X X		
- 5.50 - 6.00 - -	UT	16	60/450 mm					-	Soft to firm greyish brown slightly gravelly locally very sandy silty CLAY. Sand is fine and medium. Gravel is angular to subrounded fine and medium of limestone and quartzite.  [ESTUARINE DEPOSITS]	5.50		X   X		
- 6.00 - 6.50 6.00 - 6.50	B D	17 18						6		(1.20)		X_^_ X X X X X X		
- 6.50 - 6.95 6.50 - 7.00 6.50 - 6.95	D D SPT	19 20	N = 34 (S)					-	Dense greyish brown very silty SAND. Sand is fine and medium. [ESTUARINE DEPOSITS]	6.70		XX X XX XX XX		
7.00 - 7.50 7.00 - 7.50	B D	21 22						7 -	Soft to firm slightly gravelly silty CLAY. Gravel is subangular and subrounded fine and medium of limestone and quartzite. [ESTUARINE DEPOSITS]	7.00		× × × × × × × × × × × × × × × × × × ×		
- 7.50 - 8.00 7.50 - 8.00 7.50 - 8.00	B D UT#	25 24 23	80/ mm					-	Soft brown slightly sandy gravelly CLAY with low cobble content. Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of limestone, quartzite and psammite. Cobbles (<100mm x 100mm x 120mm) are subangular	7.50		× × × × × × × × × × × × × × × × × × ×	-	
- 8.00 - 8.50 8.00 - 8.50	B D	26 27						8 —	and subrounded of limestone. [ESTUARINE DEPOSITS]  Very dense dark brownish grey clayey very sandy GRAVEL with frequent pockets of stiff brownish grey slightly sandy gravelly clay. Sand is fine to coarse. Gravel is angular and	8.00				
- - 8.50 - 8.95 - 8.50 - 8.95	D SPT	28	N = 45 (S)					-	subrounded fine to coarse of limestone, quartzite and psammite. [ESTUARINE DEPOSITS]	(0.70)				
9.00 - 9.50 9.00 - 9.50	B D	29 30						9 —	Very stiff dark grey slightly sandy gravelly CLAY with occasional fine and medium grey sand lenses (<200mm). Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of limestone, psammite and quartzite. [PORT CLAY]	8.70				
- - 9.50 - 9.95 9.50 - 9.95	D SPT	31	N = 34 (S)					-						
-10.00 - 10.50 10.00 - 10.50 Notes	B D	32 33						-	Continued next page			* * * * * * * * * * * * * * * * * * * *	-	

		Con	tract Name	Dubl	in Po	rt MP	2_3F	=M		Locatior	n ID			
-fig	RO	Clier	nt	Dubl	in Po	rt Co	mpar	าง		3FN	и і	RH.	S.	.02
			ro Reference	F211				-,		<b>O</b> 1 1	··—	<b>J</b> 11	J	U L
•			rdinates (m)						Ground Elevation (m Datum)	Sheet 3	of 6			
			e Type	Cabl	e Pei	cuss	ion to	Rota		Status		Preli	mina	ry
Depth	Sampl		nd In Situ Testing			ecove			Strata Details				Grou	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
			results	(,0)	(70)	()	( )	. ,		()		* • •		
- - - 10.50 - 10.88 - 10.50 - 10.88 -	D SPT	34	50/226 mm (S)					-		(3.30)				
- 	В	35						11 —						
- 11.50 - 11.91 11.50 - 11.92 -	D SPT	36	50/265 mm (S)					-						
-12.00 - 12.50 - -	В	37						12 —	Very stiff dark grey slightly sandy gravelly CLAY with low cobble content and with occasional fine and medium grey sand lenses (<200mm). Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of limestone, psammite and quartzite. Cobbles (<90mm x 70mm) are angular of dark grey limestone.	12.00				
- 12.50 - 12.82 12.50 - 12.82 - - - -13.00 - 13.50	D SPT	38	50/175 mm (S)					13 —	PORT CLAY]					
- - 13.50 - 13.81 13.50 - 13.81 -	D SPT	40	50/160 mm (S)					-		(2.40)				
- 14.00 - 14.40 - -	В	41						14 —						
- 14.40 - 14.56 - - 14.40 - 15.10	SPT		50/65 mm (C)	71	0	0		-	Very dense dark grey slightly sandy clayey GRAVEL with low cobble content and with rare seams (<70mm) of firm dark grey sandy gravelly clay. Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of limestone, psammite. Cobbles (<140mm x 90mm) are dark grey and greenish grey angular to subrounded of strong dark grey limestone.	14.40				
									Continued next page					
Notes														
- Abbreviations	and res	ults da	ta defined on 'Note	s on E	xplor	atory I	Positi	on Re	cords'					

07/11/2022

Print Date

		Con	tract Name	Dubl	in Po	rt MP	2_3F	М		Location	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	ny		3FN	VI E	3H-	S-	02l
				F211	210						_			_
			rdinates (m) Type	Cabl	o Por	roucci	ion to	Pote	Ground Elevation (m Datum)	Sheet 4 Status	of 6	Droli	minary	,
	0	_						TOLE	,	Olalus		i ieii		
Depth	Sampi	ing an	nd In Situ Testing			ecove	ery		Strata Details				Groun	dwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / nstallation
- 15.10 - 15.40 15.10 - 15.40	D SPT	42	100 mm 50/150 mm (S)					-	[GLACIAL DEPOSITS] 14.40m to 14.60m; assumed zone of core loss. 15.10m to 15.20m; very stiff gravelly slightly sandy clay Sand is fine to coarse. Gravel is angular to subrounded fine and medium of limestone.					
- _ 15.10 - 15.85 - -				87	0	0		- - -	45.05   40.00	(2.20)				
- - - 15.85 - 16.60				80	0	0		16 — - -	15.85m to 16.00m; assumed zone of core loss.  16.00m to 16.10m; very closely spaced subhorizontal calcite veins (<2mm).					
- - 16.60 - 16.74 -	SPT		50/60 mm (S)					- - -	Assumed zone of core loss. 16.60m to 16.74m; SPT no recovery.	16.60		*****		
- - - 16.60 - 17.60 - -				0	0	0		17 — - - -		(1.75)				
17.60 - 18.10	D	43	80 mm	0	0	0		18 —						
- 18.10 - 18.22 18.10 - 18.23 - - - - 18.10 - 18.85	SPT	43	50/50 mm (S)	89	0	0		- - - -	18.10m to 18.18m; very stiff gravelly slightly sandy clay. Sand is fine to coarse. Gravel is angular to subrounded fine and medium of limestone.  18.18m to 18.35m; assumed zone of core loss.  Very dense dark grey slightly sandy clayey GRAVEL with low cobble content with occasional pockets (~20mm) of firm dark grey sandy gravelly clay. Sand is fine to coarse. Gravel is angular to subrounded fine and medium of limestone. Cobbles (110mm x 80mm) are angular to subrounded strong dark	18.35				
- - 18.85 - 19.60				0	0	0		- 19 — - -	grey and grey limestone [GLACIAL DEPOSITS] Assumed zone of core loss.	(0.75)		***************************************		
- 19.60 - 19.73 - 19.80 - 19.90 - 19.80 - 20.10	SPT C	44	50/50 mm (S)				3	- - - - -	Medium strong to strong locally weak dark grey LIMESTONE locally with extremely closely spaced inclined (80-90°) calcite veins (<4mm). Fresh locally distinctly weathered to destructured. Discontinuities; Set#1, bedding	19.60				
Notes - Abbreviations	and res	ults da	ta defined on 'Note:	s on E	Explora	atory F	Positio	on Red					<u> </u>	

Print Date

		Cont	ract Name	Dubl	in Po	rt MP	2_3F	M		Location	ı ID			
-fug	RO	Clier	 nt	Dubl	in Po	rt Coi	mpar	ny		3FN	<b>И_В</b>	iН.	S-	.02
		Fugr	o Reference	F211	210					0	··			<b>-</b>
•		Coor	dinates (m)						Ground Elevation (m Datum)	Sheet 5	of 6			
		Hole	Туре	Cabl	e Per	cussi	ion to	Rota	ary Coring	Status		Preli	minar	у
Depth	Sampl	ing an	d In Situ Testing	Co	re Re	ecove	ery		Strata Details				Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	.egend	Water Strike	Backfill / Installation
- 20.20 - 20.40 19.60 - 21.10 - 20.50 - 20.66				87	79	21	15	- - - -	plane fractures, inclined (0-10°), very closely to closely spaced, planar, smooth very tight, clean. [LIMESTONE BEDROCK] 19.60m to 19.80m; assumed zone of core loss. 20.10m to 20.20m; weak. Distinctly weathered to destructured with angular and subangular fine and medium gravel-sized lithorelicts. 20.40m to 20.50m; weak. Distinctly weathered to destructured with angular and subangular fine and medium gravel-sized lithorelicts.	(1.50)				
								-	20.66m to 20.76m; non-intact. Recovered as			$\Box$		
20.66 - 20.76 20.76 - 20.89	С	45					-	-	angular and subangular fine to coarse gravel sized fragments.			井		
20.76 - 21.10							12	21	Medium strong to strong locally moderately weak and weak grey and dark grey LIMESTONE locally with frequent to abundant	21.10				
21.25 - 21.50 21.40 - 21.50 -	С	46					12	- - -	fossil fragments (<5mm) including crinoids. Fresh locally distinctly weathered and destructured. Discontinuities; Set#1, bedding plane fractures, incline (0-10°), extremely closely spaced to closely spaced, planar smooth, very tight, clean. Set#2, joints, inclined (80-90°), planar, smooth, very tight, calcite					
21.10 - 22.60	С	47		90	39	0		22 —	mineralisation (<2mm). [LIMESTONE BEDROCK] 21.10m to 21.25m; assumed zone of core loss. 21.50m to 21.94m; moderately strong. Distinctly weathered with thin laminations to extremely closely spaced bedding plane fractures. 21.94m to 22.17m; weak. Destructured with angular and subangular fine and medium gravelsized lithorelicts. 22.17m to 22.60m; inclined (85°) joint with calcite mineralisation (<2mm). 22.20m to 22.60m; inclined (80°) calcite vein (<20mm).	(2.04)				
22.17 - 23.14	С	48					8	-	22.60m to 23.14m; frequent to abundant white fossil fragments (<5mm) including crinoids and with an inclined (80°) calcite vein (<10mm).					
23.14 - 23.18	D C	49 50			00	-1			Medium strong to strong locally very weak to weak dark grey locally grey LIMESTONE with rare white fossil fragments (<2mm). Fresh	23.14				
22.60 - 24.10 - 23.26 - 23.74 				100	68	51	4	- - - - - 24 —	locally distinctly weathered and destructured. Discontinuities; Set#1, bedding plane fractures, incline (0-10°), extremely closely spaced to medium spaced, planar smooth, very tight, clean. locally woth clay smear (<0.5mm). Set#2, joints, inclined (80-90°), widely spaced, planar and undulating, smooth, very tight, clean. [LIMESTONE BEDROCK] 23.14m to 23.26m; very weak to weak. Destructured with angular to subangular fine and medium gravel-sized lithorelicts. 23.74m to 24.00m; distinctly weathered to					
24.47 - 24.60 24.50 - 24.63 24.63 - 24.67	С	51		100	65	35	8 -	- - - - -	destructured with angular and subangular fine to coarse gravel-sized lithorelicts.  24.00m to 24.50m; very weak to weak. Distinctly weathered with extremely closely spaced bedding plane fractured to destructured with angular to subangular fine and medium gravel-sized lithorelicts.  24.63m to 24.67m; non-intact. Recovered as angular fine and medium gravel sized fragments.					
24.67 - 25.18 _24.98 - 25.09	С	52		.50	33	, ,,	6	_	Continued next page					
Notes	and rec	ulte da	ta defined on 'Note	e on E	volor	oton, [	Pocitio	on Do	corde'					

07/11/2022

Print Date

		Cont	tract Name	Dubl	in Po	rt MF	P2_3F	=M		Locatio	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	าy		3FI	VI F	3H-	-S-	02
			o Reference	F211	210									<b>-</b>
·		_	rdinates (m)	Cabi	a Da		ion to	. Dat	Ground Elevation (m Datum)	Sheet 6	of 6	Deali		
	Samn		Type  Id In Situ Testing		ore R			Rola	ary Coring Strata Details	Status		Preii	Groun	y ndwater
Depth (m)	Samp	iiig aii	Test			1	_		T	Depth	T			
,	Туре	No.	Results	(%)	SCR (%)	(%)	(-)	Depth (m)	Strata Descriptions	(Thickness (m)	) Level (m Datum)	Legend	Water Strike	Backfill / Installation
_ _ 25.18 - 25.35 - -							-		25.18m to 25.35m; non-intact to partially intact. Recovered as angular fine and medium gravel- sized fragments.	(4.01)			-	
25.35 - 25.77 25.60 - 25.77	С	53					5						-	
25.77 - 26.00 - - 25.60 - 27.10				100	75	37	48	- 26 — -	26.00m to 26.45m; incipient fracture at 70 degrees planar.  26.15m to 26.45m; inclined (80-90°) calcite vein (<10mm).				-	
26.00 - 26.78 - 26.57 - 26.80	С	54		.00			8	-					-	
26.78 - 26.90 - - 26.90 - 27.15							- 8	27 —	26.78m to 26.90m; non-intact. Recovered as angular and subangular fine to coarse gravel-sized fragments.				-	
_ - _ 27.15 - 27.80							11		27.10m to 27.15m; 50mm gain of limestone bedrock.  Strong grey and dark grey fine grained shelly LIMESTONE with abundant shell fossils (<5mm), predominately crinoids. Fresh. Discontinuities; Set#1, bedding plane fractures				-	
- 27.59 - 27.80 - 27.10 - 28.60	С	55		100	75	35			inclined (0-10°) very closely to closely spaced, planar, smooth and rough, tight and very tight, clean or with slight grey staining. [LIMESTONE BEDROCK]  Medium strong locally moderately weak dark				-	
_27.97 - 28.06 _ 27.95 - 28.22	С	56					11	28 -	grey to black LIMESTONE. Slightly weathered locally distinctly weathered to destructured. Discontinuities; Set#1, bedding plane fractures inclined (0-10"), extremely closely spaced to closely spaced, planar smooth, very tight, clean. [LIMESTONE BEDROCK] 27.80m to 27.95m; weak to moderately weak. Distinctly weathered with extremely closely spaced	(0.80)			-	
28.22 - 28.65 - 28.47 - 28.53	С	57					23	-	bedding plane fractures to destructured with angular fine and medium gravel-sized fragments.,  End of Borehole at 28.60 m	28.60			-	
-								29 —						
– Notes								-						
	and res	sults da	ta defined on 'Note	s on E	Explor	atory	Positi	on Re	cords'					
Template: FGSL/HI	BSI/FGSL (	Combined	CP+RC FI.hbt/Config Fu	gro Rev	5/23/12	/2019/T	S+AW			Print Date		07/11/2	2022	

		Contract Na	ame	Dub	olin Port MP2_3FM						Locati	ion ID	
-fug	RO	Client		Dub	olin Port Company						3F	M_BH	I-S-02
		Fugro Refe	rence	F211	1210								
•		Coordinate	s (m)			Groun	d Elevation (n	n Dati	um)		Sheet	1 of 1	
		Hole Type		Cab	le Percussion and Rota	y Corir	ng				Status	s Pr	eliminary
					Standard Penet	ration 1							
Test Depth (m)	Test Ty	/pe Self Wei	ght Test	Resul	lt		Total Penetration (mm)	Ham N	mer Serial lumber	Energy	Ratio (%)	Casing Depth (m)	Water Depth (m)
Test Depth (m) 0.50 2.50 4.50 6.50 8.50 9.50 10.50 11.50 12.50 13.50 14.40 15.10 16.60 18.10 19.60	Test Ty S S S S S S S S S S S S S S S S S S S	Penetration	N=0 N=0 N=4 N=3: N=4: N=5: N=5: N=5: N=5: N=5: N=5: N=5: N=5	(0,0/0 (0,0/0 (0,0/0 4 (4,9/) 5 (5,7/) 4 (4,9/) 0 (9,12 0 (7,6/) 0 (5,7/) 0 (25 f 0 (25 f 0 (25 f	It (1,0,0,0) (1,0,0,0) (1,0,0,0) (1,0,0,0) (1,0,0,0) (1,0,0,1,3) (1,0,9,9,8) (1,1,1,1,4) (1,1,1,4) (1,1,1,4) (			AF AF AF AF AF AF AF AF AF AF	Table 1		Ratio (%) 54 54 54 54 54 54 54 54 54 54 54 54 55 54 55 54 55 55	Casing Depth (m)  0.50 2.50 4.50 6.50 8.50 9.50 10.50 11.50 12.50 13.50 14.40 15.10 16.60 18.10 19.60	Water Depth (m  0.00  0.00  0.00  0.00  0.00  0.00  0.00  DRY  DRY  DRY  DRY  O.00  0.00  0.00  0.00
In Test Depth (m) Test	Une Un	nne Test Res disturbed Undrained lear Strength (kPa)	ults Residual Unc Shear Strengi		In Situ Hand Per Test Depth (m)		eter Results ed Undrained Shear S (kPa)	trength		Heads <sub>k</sub>		ing by Photoionis	sation Detector

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL SPT Summary.hbt/Config Fugro Rev5/18/02/2019/TS

Print Date

			Со	ntract	Nam	е	Dublin	n Port MI	P2_3	FM									Locat	ion II	D		
-6	lGF		Cli	ent		ı	Dublin	n Port Co	nmna	nv.									<b>3</b> E	M	R	H_9	S-03
				gro Re	eferer		=2112		Jilipo	u i y									JI	IVI		11-4	J-UJ
				ordina				152.50 N	7341	125 (	00	Gro	und	Elevation	(m D	atur	n) 0.	79	Sheet	t 1 of	1		
			-	le Typ				Percus							,		.,  0.		Status		-	Prelim	inarv
			1	٠,٢				2230		_		iipme	Ť										,
Depth From (m)	Depth To (	n) Ho	ole Type	Date F	rom	Date To	Е	Equipment	С	ore Ba	_ <u> </u>	Core		Drilling Crew	Logge	d By	Remark	S					
0.00	16.81	$\top$	CP	09/11/2	2022	09/11/2022		Comacchio S1200, Pilcor	n		7			CR, AB, JW, SB	FS,								
16.50	29.00		RC	10/11/2	2022	10/11/2022	1	Wayfarer cchio MCS12						CR, AB	FS,	sı							
. 5.55	20.00			, 11/2				0012						,	. 5,								
					rogre									Ro	tary [							ore De	etails
Date (dd/mm/yyyy	Tir ) (hh:	nm)	Hole D (m	)	(m)	Water Depth (m)	vvcaui				Dep From	(m)	pth To (m)	Flush Typ	ре	(	70)	Flush Colou	(nn.i	mm)	Depth From (m)	Depth To (m)	Diameter (mm)
09/11/2022 09/11/2022	2 11:		0.0 12.9		0.00 12.95		Windy, Windy,				16.5 16.5		6.50 7.50	W W			0	Clear Grey	00:		16.50 17.50	17.50 19.00	102 102
09/11/2022 10/11/2022	19	00 00	12.5 16.8	50 31	12.50 16.50		"				17.5 19.0	50 1 00 2	9.00 0.50	W W		1 1	00	Grey Grey	00:	:12 :11	19.00 20.50	20.50 22.00	102 102
10/11/2022 10/11/2022	2 00	01	16.5 29.0	50	16.50 16.50						20.5	50 2	2.00 2.75	W W		1	00	Grey Grey	00:	:05 :05	22.00 22.75	22.75 23.50	102 102
	"										22.5	75 2	3.50 5.00	w w		1	00	Black Black	00:	:13	23.50 25.00	25.00 26.50	102 102
											25.0 26.5	00 2	6.50 8.00	w w		1	00	Black Black	00:	:16	26.50 28.00	28.00 29.00	102 102 102
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L										_													
				Hole a	and C	asing	•																
Depth		Н		neter (mm	1)	Depth To (	m)	Casing Dia		(mm)													
12. 16.	81		20	00 00		12.95 15.80		2	200 200														
29.				46		16.50			200														
								L		_													
			Chis	elling	/ Slov	v Progres	ss																
Depth F		$\mathbf{I}$		To (m)		Duration (hh:	mm)	Tool /	Remar	k													
15.	80	$\top$	16	.30		02:00																	
				Strike	;			Water															
Strike At (m)	Rise To (r	n) Tir	ne Elapse (mins)	<sup>Q</sup> Casin	g Depth (	m) Depth Se	ealed (m)	Depth From (m)	Dep (r	th To m)													
				<u> </u>				L ,															
Groundwate	er observa			Strike f		irks		-	Rorehal	e advic	nced .	ieina o-	ble ac	roussion with -				narks	tecting (	SPT\ C	Client inct	ructed rate	any switch of
J. Sunuwalt	52561 vd			. 2 0 FG1 W	J. 101.									rcussion with sa at 29.00m metr					esting (	or1). (	Jilent Inst	i uciea rota	ary switch at
			Inct-II	otio:-								Di				_				Deel	fill		
Tomas	Tip Dept	h/ F	Install Response	Zone Res	sponse 7	one Installation	n Dot-	ID	Top Dep	th / \		Pipe Depth (m	\ B:-	neter (mm)	Tues	+	epth From	(m) D==#-		Back	till ackfill Mat	orio!	Date
Туре	Distance	(m)	Top (m	1)	Base (m)	metaliation	Date	טו	.op Dep	-sir (III)	⊔ase	Sehri (III	, Dian	1000 (IIIII)	Туре	106	0.00	(m) Depth 1		Ба	Grout	unal	10/11/2022
Notos																							
Notes	ations -	nd	aculta :	tata da	fined	in 'Explora	aton, I	ocation 5	2000-	de V-	wah	oetc'											
- Applevi	auons 8	iiu 16	ะอนแร (	uala UE	mieu	iii Explora	atory L	Location F	VECOL	us Ne	ysil	ಆರเಶ											
Charles							1-	lovoti D :	unc		Tr	iblic D	+ C-	t Dotu		T_	rid Cr	lingte Cont	<u> </u>	TM			
Checked By		/ECO:	DLI C	mentil	HC*	Fuers D		levation Dat	um		Du	INIIU HOL	ı Unar	t Datum		Gi	iu Coord	linate Syster		TM to		14/44/00	22
remplate: F	GSL/HBSI	/FGSL	. BH Sun	nmary.hbt	t/Config	Fugro Rev5/	26/06/20	J19/TS+AW											Print Da	te		14/11/202	22

		Con	tract Name	Dubl	in Po	rt MF	P2_3I	FM		Location	n ID			
-fug	RO	Clie	nt	Dubl	in Po	rt Co	mpai	ny		3FI	И	BH.	<b>-S-</b> (	03
		Fugi	ro Reference	F211	210						· · — ·			
•			rdinates (m)	_				25.00	, ,	Sheet 1	of 6	I=		
	1	Hole	Туре	Cabl	e Pe	rcuss	ion to	o Rota	ary Coring	Status		Preli	minary	
Depth	Sampl	ing ar	nd In Situ Testing	Co	ore R	ecove	ery		Strata Details		1	1	Ground	dwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum	Legend	Water Strike	Backfill / nstallation
0.00 - 0.50 0.00 - 0.50	B D	1 2						-	Very soft to soft black slightly sandy silty CLAY with occasional shells (<40mm x 30mm) and strong sulphurous odour. Sand is fine and medium. [RECENT ESTUARINE DEPOSITS]			× × × ×		
0.50 - 0.95 0.50 - 1.50 0.50 - 0.95	D B SPT	3 4	0/450 (S)					-		(1.00)		× × ×		
-								1-	Black slightly sandy very clayey GRAVEL with rare shells (<75mm x 50mm) and strong sulphurous odour. Sand is fine. Gravel is angular and subangular fine and medium of	1.00	-0.21	× × × × × × × × × × × × × × × × × × ×		
1.50 - 1.95	D SPT	5	N 40 (0)					-	limestone and sandstone. [RECENT ESTUARINE DEPOSITS]  Soft black silty CLAY with strong sulphurous	(0.50)	-0.71	× ×		
1.50 - 1.95	581		N = 13 (S)					-	odour. [RECENT ESTUARINE ĎEPÖSITS]	(0.30)	-1.01	×		
- 2.00 - 2.50	В	6						2-	Medium dense dark grey slightly sandy GRAVEL with frequent pockets of firm brown very sandy clay and strong hydrocarbon odour. Sand is medium. Gravel is angular to rounded fine to coarse of limestone sandstone calcite	(0.70)	-1.01			
2.50 - 2.95	D	7	N. 40/0					-	and quartz. [ESTUARINE DEPOSITS]  Medium dense dark grey silty SAND. Sand is	2.50	-1.71	× ×		
2.50 - 2.95	SPT		N = 10 (S)					-	fine and medium. [ESTUARINE DEPOSITS]	(0.50)		X X X X X X X X X X X X X X X X X X X		
- 3.00 - 3.50	В	8						3	Dark grey sandy GRAVEL with occasional pockets of soft clay (<70mm x 50mm x 40mm) and with 1 No. wood fragment (<95mm x 10mm x 5mm) and with strong hydrocarbon odour.	3.00	-2.21	××		
3.50 - 3.95 3.50 - 3.95	D SPT	9	N = 4 (S)					-	Sand is medium and coarse. Gravel is subangular fine of limestone and sandstone. [ESTUARINE DEPOSITS]  Loose dark grey silty SAND. Sand is fine and	3.50	-2.71	××>		
								-	medium, predominantly fine. [ESTUARINE DEPOSITS]	(0.50)		× × × × × × × ×		
- 4.00 - 4.50	D	10						4	Dark grey slightly clayey sandy GRAVEL. Sand is medium and coarse. Gravel is subangular fine of limestone and sandstone. [ESTUARINE DEPOSITS]	4.00	-3.21	^× ×		
4.50 - 4.95 4.50 - 5.50 4.50 - 4.95	D B SPT	11 12	N = 12 (S)					-	Medium dense dark grey very silty SAND with occasional shells (<50mm x 50mm) and shell	4.50	-3.71	× × × ×		
								-	fragments (<5mm x 2mm) rare wood fragments (<15mm x 10mm x 5mm) and strong sulphurous odour. Sand is fine and medium, predominantly fine. [ESTUARINE DEPOSITS]			X X X X X X X X X X X X X X X X X X X		
-								-	Continued next page	7		×	ľ	<u> XXXXX</u>

		Cont	tract Name	Dubl	in Po	rt MP	<sup>2</sup> _3F	-M		Locatio	n ID			
-fuc	RO	Clier	nt	Dubl	in Po	rt Co	mpar	ny		3FI	M I	3H.	S-	03
			o Reference	F211	210					<b>∃</b> 0	··—			•
•			rdinates (m)	E718	3452.	50 N	7341	25.00	Ground Elevation (m Datum) 0.79	Sheet 2	of 6			
		Hole	Туре	Cabl	e Per	cuss	ion to	Rota	ary Coring	Status		Preli	minaı	у
Depth	Samp	ling an	d In Situ Testing	Co	ore R	ecove	ery		Strata Details				Groui	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
- 5.50 - 5.95 5.50 - 6.50 5.50 - 5.95	D B SPT	13 14	N = 2 (S)					- - - -		(1.50)		X X X X X X X X X X X X X X X X X X X		
-								6 —	Soft dark grey slightly sandy slightly gravelly CLAY with strong sulphurous odour. Gravel is angular to subrounded fine to coarse of limestone. [ESTUARINE DEPOSITS]	6.00	-5.21			
- 6.50 - 7.50 -	PS	15						-	Soft brownish grey silty CLAY with strong sulphurous odour. [ESTUARINE DEPOSITS]	6.50	-5.71	  		
- - -								7 —		(1.00)		X X X X X X X X X X X X X X X X X X X		
- 7.50 - 7.95 7.50 - 8.50 7.50 - 7.95	D B SPT	16 17	0/450 (S)					-	Firm brownish grey slightly sandy silty CLAY with rare wood fragments (<5mm x 4mm x 1mm) and strong sulphurous odour. Sand is fine. [ESTUARINE DEPOSITS]	7.50	-6.71	× × × × × × × × × × × × × × × × × × ×		
- - -								8 —	Firm brownish grey gravelly CLAY with occasional wood fragments (<10mm x 5mm x 2mm) and occasional shell fragments (<10mm x 2mm) with moderate sulphurous odour. Gravel is subangular and subrounded fine to coarse of sandstone and limestone.	8.00	-7.21	×		
- 8.50 - 8.95 8.50 - 9.00 8.50 - 8.95	D B SPT	18 19	N = 17 (S)					-	[ESTUARINE DEPOSITS]  Medium dense grey slightly sandy GRAVEL. Sand is medium and coarse. Gravel is subangular to rounded fine to coarse of limestone sandstone quartz and calcite. [ESTUARINE DEPOSITS]	8.50	-7.71			
- - 9.00 - 9.50 -	В	20						9 —	Grey very sandy GRAVEL. Sand is medium and coarse. Gravel is subangular to rounded fine to coarse of limestone sandstone psammit and calcite. [ESTUARINE DEPOSITS]	9.00 ee (0.50)	-8.21			
- 9.50 - 10.00 9.50 - 9.95 9.50 - 9.95	B D SPT	22 21	N = 29 (S)					-	Medium dense grey slightly clayey slightly sandy GRAVEL. Sand is medium and coarse. Gravel is subangular and subrounded fine and medium of limestone sandstone and calcite. [ESTUARINE DEPOSITS]	9.50	-8.71			
- —10.00 - 10.50	В	23						-	Continued next page	10.00	-9.21			
Notes			I						l		<u> </u>			

		Con	tract Name	Dubl	in Po	rt MP	<sup>2</sup> _3F	-М		Location	ı ID		
-fug	RO	Clie	nt	Dubl	in Po	rt Co	mpar	าy		3FN	Л	BH-	S-03
		Fugi	ro Reference	F211						• •	··—		
Y			rdinates (m)	_				25.00	, ,	Sheet 3	of 6		
		Hole	е Туре	Cabl	e Pei	rcussi	ion to	Rota	ary Coring	Status		Preli	minary
Depth	Samp	ling ar	nd In Situ Testing	Co	ore R	ecove	ery		Strata Details				Groundwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)		FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum	Legend	Water Backfill / Installation
-								-	Dark grey sandy very clayey GRAVEL with low cobble content. Gravel is subangular to rounded fine to coarse of limestone. Cobbles (<160mm x 75mm x 30mm) are subangular of limestone. [ESTUARINE DEPOSITS]	(0.50)			
- 10.50 - 10.95 10.50 - 11.00 10.50 - 10.95	D B SPT	24 25	N = 25 (S)					-	Medium dense dark grey clayey sandy GRAVEL. Sand is medium and coarse. Gravel is subangular and subrounded fine to coarse of limestone and sandstone. [ESTUARINE DEPOSITS]	10.50	-9.71		
- 11.00 - 11.50 -	В	26						11 —	Dark grey slightly silty gravelly SAND with 1 No cobble and with rare pockets of sandy clay (<85mm x 60mm x 40mm). Sand is fine to	11.00	-10.21		
- - - 11.50 - 11.95	D	27						-	coarse. Gravel is angular to subrounded fine to coarse of limestone sandstone quartz. Cobble (<100mm x 75mm x 50mm) subrounded of limestone. [ESTUARINE DEPOSITS]	(0.50)	-10.71		
11.50 - 11.95 11.50 - 12.00 11.50 - 11.95	B SPT	28	N = 34 (S)					-	Dense dark grey very sandy GRAVEL with frequent pockets of firm brownish grey clay. Sand is medium and coarse. Gravel is subangular to rounded fine to coarse of limestone sandstone and calcite. [ESTUARINE DEPOSITS]	11.50	-10.71		
12.00 - 12.50 - -	В	29						12 —		(1.00)			
- 12.50 - 12.95 12.50 - 13.00 12.50 - 12.95	D B SPT	30 31	N = 43 (S)					-	Dense dark brown slightly clayey SAND. Sand is medium and coarse. [ESTUARINE DEPOSITS]	12.50	-11.71		
_ -13.00 - 13.50	В	33						13 —	12.80m to 12.95m; stiff dark brown slightly sandy clay. Sand is fine.	(0.50)	-12.21		
13.00 - 13.50	D	32						-	Very stiff dark grey slightly gravelly sandy CLAY with low cobble content. Sand is fine to coarse. Gravel is subangular and subrounded fine to coarse of dark grey limestone and siltstone. Cobbles (<80mm x 90mm x 120mm) are subrounded of limestone. [PORT CLAY]				
- 13.50 - 13.95 13.50 - 13.95	D SPT	34	N = 40 (S)					-	13.50m to 13.60m; dark grey and grey fine to coarse sand.				
14.00 - 14.50 - -	В	35						14 —					
- 14.50 - 14.92 14.50 - 14.92	D SPT	36	50/265 mm (S)					-		(3.50)			
- -15.00 - 15.50	В	37						_	Continued next page				
Notos									Continued next bage				

		Con	tract Name	Dubl	in Po	rt MP	2 3F	-M		Locatio	n ID			
-fire	RO	Clier	nt			rt Co				3FI	и в	RН.	S	<b>.</b> 03
1-4			o Reference	F211		11.00	pui	.,		J	<b>'</b> '-'	JI 1º	- <b>O</b> -	00
			rdinates (m)			50 N	7341	25.00	Ground Elevation (m Datum) 0.79	Sheet 4	of 6			
			Туре	_					ary Coring	Status		Preli	minar	γ
5 "	Samp		d In Situ Testing			ecove			Strata Details	1			Grour	ndwater
Depth (m)	Туре	No.	Test		SCR		FI	Depth	Strata Descriptions	Depth (Thickness	Level	Legend	Water	Backfill /
	1,400	140.	Results	(%)	(%)	(%)	(-)	(m)	·	(m)	(m Datum)	Logona	Strike	Installation
- 15.50 - 15.90	D	38						-	15.00m to 15.20m; dark grey subangular and subrounded fine to coarse gravel of limestone and siltstone.					
-16.00 - 16.50 16.00 - 16.50	SPT B D	39 40	50/245 mm (S)					- - - - 16 —	16.00m to 16.50m; dark grey and brownish grey subangular and subrounded fine to coarse gravel					
- 16.50 - 16.81	SPT		50/160 mm (S)					-	of limestone and siltstone.  Assumed zone of core loss.	16.50	-15.71			
- - -17.00 - 17.25 - 16.50 - 17.50	В	41		70	0	0		17 —	Very dense dark grey to brownish grey sandy clayey GRAVEL with low cobble content. Grave is subangular and subrounded fine to coarse or dark grey limestone siltstone and subrounded white calcite. Cobbles (<60mm x 100mm x 120mm) are subrounded of dark grey limestone. [GLACIAL DEPOSITS].		-16.01			
- - 17.50 - 17.80 - -	SPT		50/151 mm (S)					-	17.80m to 18.20m; assumed zone of core loss.	(1.40)				
18.20 - 18.55 17.50 - 19.00	В	42		53	0	0		18 —	Dark grey and brownish grey slightly clayey GRAVEL. Gravel is subangular to subrounded	18.20	-17.41			
- - -								-	fine to coarse of dark grey limestone, brownish grey siltstone and possible schist. [GLACIAL DEPOSITS].  Dark grey slightly clayey slightly sandy gravelly COBBLES. Gravel is subangular and subrounded medium to coarse of limestone.  Cobbles (<80mm x 90mm x 120mm) are subrounded dark grey limestone with frequent calcite veins (<3mm). [GLACIAL DEPOSITS].  18.93m to 19.00m; very clayey gravel.	18.55	-17.76			
19.40 - 19.70	В	43							Dark grey to brownish grey slightly clayey GRAVEL with low cobbles content. Gravel is subangular and subrounded fine to coarse of dark grey limestone, siltstone, white calcite and greenish grey possible schist. Cobbles are (<50mm x 65mm x 100mm) of subrounded dar grey limestone. [GLACIAL DEPOSITS].	(0.70)	-18.21			
19.00 - 20.50				47	0	0		-	19.60m to 19.70m; very clayey gravel.  Assumed zone of core loss.	19.70	-18.91			
_								-	Continued next page				1	·xxxxx
Votes								1		<u> </u>				

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Cont	tract Name	Dubl	in Po	rt MP	2_3F	M	ļi.	_ocatior	טו ו			
-fire	RO	Clier		Dubl	in Po	rt Co	mpar	1V		3FN	ЛБ	RH.	S	<u>.</u> ევ
			o Reference	F211		11 00	пра	.,		JI I	"— <b>'</b>	JI 1.	<b>-</b>	.00
•			rdinates (m)	_	_	50 N	7341	25.00	Ground Elevation (m Datum) 0.79	Sheet 5	of 6			
		_	Type	_					` '	Status	01 0	Preli	mina	
		I IOIC	Туре	Cabi	e i ei	cuss	OII to	INOL	ary Coming [	Jialus		i ieii	IIIIIIai	У
Depth	Sampl	ing an	nd In Situ Testing	Co	re R	ecove	ery		Strata Details		ı		Grou	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
20.50 - 22.00				0	0	0		21		(3.05)				
22.75 - 23.10 - 23.10 - 23.20 22.75 - 23.50 23.10 - 23.25 23.25 - 23.60	С	44	100 mm	100	15	0	13	23 —	Strong dark grey LIMESTONE. Fresh to slightly weathered. Predominately non-intact, recovered as angular and subangular medium to coarse gravel-sized fragments.  Discontinuities; Set#1, bedding plane fractures, inclined (0-5°), very closely to closely spaced, vert tight to tight, planar and undulating, smooth, clean and with clay smears (<0.3mm). [LIMESTONE BEDROCK].	(0.75)	-21.96			
23.60 - 23.70 23.70 - 23.80 23.70 - 24.17 - 23.80 - 24.17 - 24.32 - 24.60 - 24.32 - 24.67 - 24.67 - 24.74	С	45	280 mm	95	77	40	5 - 12 - 8	24	Strong dark grey locally grey LIMESTONE.  Slightly weathered. Discontinuities; Set#1; bedding plane fractures, inclined (0-5°), very closely to closely spaced, planar and undulating, smooth and rough, tight, clean occasional to frequent clay smears and some orangish brown staining. [LIMESTONE BEDROCK]. 23.60m to 24.17m; partially non-intact along inclined (80-90°) calcite vein (<30mm).  Strong to very strong dark grey locally grey LIMESTONE. Fresh. Discontinuities; Set#1, bedding plane fractures, inclined (0-5°), very closely to medium spaced, planar and undulating, smooth and rough, very tight to partially open, clean and with occasional to frequent orangish brown staining occasional clay smears. Set#2, joints, inclined (70-80°), planar, smooth to rough, tight, clay smear. [LIMESTONE BEDROCK] 24.17m to 24.32m; non-intact. Recovered as angular and subangular medium to coarse Continued next page	(0.67) - 24.17	-22.71			

Print Date

		Cont	ract Name	Dubl	in Po	rt MP	2_3F	М		Lo	ocation	ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	ny		3	3FN	/1_E	3H-	S-	03
				F211						4				_	
			dinates (m) Type					25.00 Rota	Ground Elevation (m Datum) 0.79  ary Coring	-	heet 6 tatus	of 6	Proli	minar	
	Samp		d In Situ Testing			ecove		rtota	Strata Details	Oi	lalus		i icii		dwater
Depth (m)	Samp	iliy ali	_						Strata Details					Giouii	uwatei
(111)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions		Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
25.00 - 25.70				100	00	90	-	- - - - -	gravel-sized fragments. 24.67m to 24.74m; non-intact. Recovered as angular and subangular medium and coarse gravel-sized fragments. 24.93m to 25.00m; assumed zone of core loss.						
26.05 - 26.40 26.05 - 26.50	С	46	350 mm	100	98	89	5	26 —							
26.50 - 26.74							20	-	26.60m to 26.72m; medium strong. Moderately weathered becoming distinctly weathered.		(4.83)				
26.86 - 27.04	С	47	180 mm					-	26.73m to 26.82m; 2 No. conjugated inclined (80-90°) calcite veins (<20mm).						
_26.74 - 27.23							10	27 —							
. 26.50 - 28.00				100	86	47		-							
27.23 - 27.55				100	00	7'	30	-							
- 27.55 - 27.59								-	27.55m to 27.59; non-intact. Recovered as dark						
27.59 - 27.68 27.68 - 27.72							10	-	grey slightly clayey angular and subangular fine to medium gravel-sized fragments. 27.68m to 27.72; non-intact. Recovered as dark						
27.72 - 28.00							11	28 —	grey slightly clayey angular and subangular fine to medium gravel-sized fragments. 27.90m to 28.28m; grey.						
28.00 - 28.28							14	-							
28.28 - 28.50 - 28.00 - 29.00				100	89	32	27	-							
28.64 - 29.04 28.50 - 29.00	С	48	410 mm				4	- 29 —	28.90m to 29.00m; inclined (70-80°) joint.		29.00	-28.21			
Notes	and rec	sulte de	ta defined on 'Neter	a on F	yplor	atory [	Onei+i-	- - - - -	End of Borehole at 29.00 m		29.00	-20.21			
- Abbreviations	and res	sults da	ta defined on 'Note	s on E	xplor	atory F	Positio	on Red	cords'						

Print Date

_	Contract Name	Dublin Port MP2_3FM				Location	ID	
<b>TUGRO</b>	Client	Dublin Port Company				3FN	1 BH	1-S-03
	Fugro Reference	F211210				]		
•	Coordinates (m)	E718452.50 N734125.00	Ground Elevation (m	Datum)	0.79	Sheet 1	of 1	
	Hole Type	Cable Percussion and Rota	ry Coring			Status	Р	reliminary
		Standard Penet	ration Test Results					
Test Denth (m) Test To	yne Self Weight Test	Result	Total Penetration	Hammer Seri	al Energy	Ratio (%) Ca	sing Denth (m	) Water Denth (m)

			Standard Penetration T					
Test Depth (m)	Test Type	Self Weight Penetration (mm)	Test Result	Total Penetration (mm)	Hammer Serial Number	Energy Ratio (%)	Casing Depth (m)	Water Depth (m)
0.50	S	0	N=0 (0,0/0,0,0,0)	450	AR3214		0.50	0.00
1.50	S	0	N=13 (0,1/3,3,3,4)	450	AR3214		1.50	0.00
2.50		0	N=10 (2,1/2,3,3,2)	450	AR3214		2.50	0.00
3.50	S S S	0	N=4 (0,0/1,2,1,0)	450	AR3214		3.50	0.00
4.50	S	0	N=12 (3,2/2,2,4,4)	450	AR3214		4.50	0.00
5.50	S S	0	N=2 (2,1/0,0,1,1)	450	AR3214		5.50	0.00
7.50	S	0	N=0 (0,0/0,0,0,0)	450	AR3214		7.50	0.00
8.50	S	0	N=17 (3,3/3,5,4,5)	450	AR3214		8.50	0.00
9.50	S	0	N=29 (8,7/7,8,7,7)	450	AR3214		9.50	0.00
10.50 11.50	S	0	N=25 (5,4/5,6,6,8) N=34 (3,4/6,7,9,12)	450 450	AR3214 AR3214		10.50 11.50	0.00 0.00
12.50	S S	0	N=43 (4,5/8,14,9,12)	450	AR3214 AR3214		12.50	0.00
13.50	S	0	N=40 (8,8/8,9,10,13)	450	AR3214		13.50	0.00
14.50	Š	ő	N=50 (6,7/50 for 265mm)	415	AR3214		14.50	0.00
15.50	S S	Ŏ	N=50 (3,8/50 for 245mm)	395	AR3214		15.50	0.00
16.50	S	0	N=50 (10,12/50 for 160mm)	310	AR3214		16.50	0.00
17.50	S	0	N=50 (10,11/50 for 151mm)	301	AR3214		17.50	0.00
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	In Situ	Vane Test Res		In Situ Hand Per	netrometer Results		e Headspace Test	ing by Photoionis	sation Detector
Test Depth (m)	Test Type	Undisturbed Undrained Shear Strength (kPa)	Residual Undrained Shear Strength (kPa)	Test Depth (m)	Undisturbed Undrained Shear Streng (kPa)	th Te:	st Depth (m)	PID Re	sult (ppm)
ł									

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL SPT Summary.hbt/Config Fugro Rev5/18/02/2019/TS

Print Date

			Со	ntract	Name		Dublin	Port MP	2_3FM									Lc	cation	ID		
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			1110	. Стуре		10	JUDIE	. Groussi										Jol	uus			iai y
Depth From (m)	Depth To (m)	Hole	Туре	Date Fr	om	Date To	E	quipment					Drilling Crew	Logge	ed By	Remar	ks					
0.00	18.72	С	P	06/11/2	022	07/11/2022	MCS	31200, Pilcon					JW, SB, CR, AB	SI, F	FS							
18.72	28.90	R	C.	07/11/2	022	07/11/2022	١ ١	Wayfarer	00				JW, SB	SI, F	FS							
															D - 1	-:				1 .	P	atail-
Date	Time	. 1	Hole D	epth Casi	ng Depth	Water Depth	Weath	er		De	pth D	epth To	1		Flus	h Return	Flush Co	lour	Run Time	Depth	Depth To	
(dd/mm/yyyy 06/11/2022	(hh:mr 2 08:30	n) O	(m) 0.00	0 (	(m) 0.00	(m)	windy	<b>0</b> 1		From 18.	n (m) .72	(m) 19.50	W	he		100	Grey	loul	(hh:mm) 00:05	From (m) 18.72	(m) 19.50	102
06/11/2022 06/11/2022	2 19:24	4	14.5	50   1	4.50		Good			20.	.00 2	20.50	W			100	Grey Grey		00:04 00:05	20.00	20.50	102 102 103
07/11/2022 07/11/2022 07/11/2022	2 07:0	0	16.5	60   1	6.50					21.	.00 2	21.50	W			100	Grey		00:04	21.00	21.50	102
07/11/2022 07/11/2022 08/11/2022	2 19:0	0	25.0	00   1	8.50					22.	.00 2	22.50	w w			100	Grey		00:05	22.00	22.50	102
08/11/2022 08/11/2022	2 09:20	0	26.5 27.4	50   1 10   1	8.50 8.50					23. 23.	.00 2	23.50 24.25	W W			100 100	Black Black	:	00:10 00:30	23.00 23.50	23.50 24.25	102 102
08/11/2022 08/11/2022	2 11:30	)	28.9	0   1	8.50					24. 25.	.25 2	25.00 26.50	w w			100 100	Black Black		00:25 00:27	24.25 25.00	25.00 26.50	102 102
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16 18	.50		20	00		15.00		20	10													
25 28	.00		14	16																		
		-	Chis	elling /	Slow	Progres	ss	1														
Depth F		_	Depth	To (m)		uration (hh:		Tool / R	temark	1												
9. 10	.90		11.	.20		01:00																
14. 16.	.90		17.	.20		01:00																
18.	.ZU		18.	UC.		U1:30																
		14	latar	Strike				Matar	۸۵۵۵													
Strike At (m)	Rise To (m)	Time	Elapse		Depth (n	n) Depth Se	aled (m)	Depth From	Depth To													
- ()	. ()	1)	illis)	9	, (	1	···/	(m)	(m)													
Crowni	or ob ''					rks																
Groundwate	observatio	ııı not r	ecorde	a over wa	uer.													on tes	ting (SPT)	. Client ins	tructed rota	ary switch at
			Full Coordinates   Full 12-10   Coordinates   Coordinate																			
		In	stalla	ation							Pipe				T				Bac	kfill		
Туре	Tip Depth of Distance (m	Re:	sponse : Top (m	Zone Resp	oonse Zo Base (m)	Installation	n Date	ID To	op Depth (m)			n) Diar	neter (mm)	Туре	D				(m) I		terial	
									<u>-</u>					_		0.00	)	25.00		Grout		07/11/2022
Notes															_1_							I
	ations an	d res	ults c	data def	fined in	n 'Explora	atory L	ocation R	ecords K	eysh	neets'											
Checked By									m	D	ublin Po	rt Char	t Datum		G	Grid Coo	rdinate Sys	_				
Template: F	GSL/HBSI/F	GSL B	H Sum	mary.hbt/	Config F	ugro Rev5/	26/06/20	)19/TS+AW										Pri	nt Date		10/11/202	22

		Cont	ract Name	Dubl	in Po	rt MP	2 3F	M		Locatio	n ID			
-fug	D۸	Clier			in Po					3FI	л г	ЗΗ.	<b>S</b> _	ŊΛ
				F211		11 001	праг	ıy		JI 1	۷ı_ı	JI 1.	. <del></del>	U+
						50 N7	7341	50.50	Ground Elevation (m Datum) 0.33	Sheet 1	of 6			
			Туре							Status		Preli	minar	у
	Samn		d In Situ Testing	Cc	re Re	acove	arv/		Strata Details	l'		<u>'</u>		ndwater
Depth (m)			Test		SCR		FI	Depth		Depth	Level			Backfill /
0.00 - 0.50	Туре	No.	Results	(%)	(%)	(%)	(-)	(m)	Strata Descriptions  Very soft dark grey silty CLAY with strong putrid	Depth (Thickness) (m)	(m Datum)	Legend	Strike	Installation
0.00 - 0.50	B D SPT B	3	0/450 (S)						Very soft dark grey silty CLAY with strong putrid odour. [RECENT ESTUARINE DEPOSITS]  Very soft black silty CLAY with strong hydrocarbon odour. [RECENT ESTUARINE DEPOSITS]	(1.50)	-1.17			
- 2.50 - 2.95 2.50 - 2.95 - 3.00 - 3.50 - 3.50 - 4.50	D SPT B	5	0/450 (S)					3—		(3.40)				
- 4.50 - 4.95 4.50 - 4.95 	D SPT B	8	N = 11 (S)					- - - - -	Dark grey and brownish grey slightly sandy  Continued next page	4.90	-4.57	X		

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Cont	tract Name	Dubl	in Po	rt MP	<sup>2</sup> _3F	-M		Locatio	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	าy		3FI	M F	3Н.	S-	04
			o Reference	F211						• • •	··—·	<b>-</b>		<b>-</b>
•		_	rdinates (m)	E718	3454.	50 N	7341	50.50	Ground Elevation (m Datum) 0.33	Sheet 2	of 6			
		Hole	Туре	Cabl	e Per	cuss	ion to	Rota	ary Coring	Status		Preli	minar	·y
Depth	Samp	ling an	nd In Situ Testing	Co	ore R	ecove	ery		Strata Details				Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
-								-	CLAY with strong putrid odour. Sand is fine. [ESTUARINE DEPOSITS]	(0.60)				
- 5.50 - 6.00 - - -	UT	10	8/500 mm					-	Soft brownish grey silty CLAY locally with occasional shell fragments (< 8mm x 5mm) and locally with rare wood fragments (<25mm x 10mm x 10mm)and with strong putrid odour. [ESTUARINE DEPOSITS]	5.50	-5.17	× × × × × ×		
- 6.00 - 6.50 - - - - - 6.50 - 6.95	В	11						6				X X X X X X X X X X X X X X X X X X X		
6.50 - 6.95 - - - - 7.00 - 7.50	SPT	13	0/450 (S)					7-	6.60m to 6.85m; occasional shell fragments (< 8mm x 5mm).	(2.50)				
- - - 7.50 - 8.00 -	UT	14	16/500 mm					- - - -	7.50m to 8.00m; rare wood fragments (<25mm x 10mm x 10mm).			X X X X X X X X X X X X X X X X X X X		
- 8.00 - 8.50 -	В	15						8 —	Firm grey CLAY with occasional wood fragments (<55mm x 40mm x 20mm) and with occasional pockets (< 80mm x 50mm x 30mm) of amorphous peat and strong putrid odour. [ESTUARINE DEPOSITS] 8.00m to 8.50m; occasional pockets (<80mm x	8.00	-7.67	× × × × × × × × × × × × × × × × × × ×		
- 8.50 - 8.95 8.50 - 9.00 8.50 - 8.95	D B SPT	16 17	N = 41 (S)					-	50mm x 30mm) of amorphous peat.	(1.00)				
- 9.00 - 9.50 -	В	18						9 —	Dark grey slightly sandy GRAVEL with medium cobble content. Sand is coarse. Gravel is angular to rounded fine to coarse of limestone. Cobbles (<130mm x 100mm x 65mm) are subangular to rounded of limestone. [ESTUARINE DEPOSITS]		-8.67			
9.50 - 9.95 9.50 - 9.95	D SPT	19	N = 25 (S)					-		(1.00)				
—10.00 - 10.50	В	20						-	Continued next page	10.00	-9.67			
Notes			1					<u> </u>			1	I		

		Cont	tract Name	Dubl	lin Po	rt MP	P2_3F	FM		Locati	on ID			
-fug	RO	Clier	nt	Dubl	lin Po	rt Co	mpar	ny		3F	<b>M_</b> I	BH.	S-	04
			o Reference	F211			•			<b>∃</b> •••				•
<b>*</b>			rdinates (m)	_				50.50		Sheet				
		Hole	Туре	Cabl	le Pei	cuss	ion to	Rota	ary Coring	Status		Preli	minar	У
Depth	Sampl	ing an	d In Situ Testing	Co	ore R	ecove	ery		Strata Details			1	Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickne (m)	ss) Level (m Datum	Legend	Water Strike	Backfill / Installation
								-	Firm dark grey slightly sandy slightly gravelly CLAY. Sand is medium. Gravel is angular to subrounded fine of limestone. [ESTUARINE DEPOSITS]	(0.50				
- 10.50 - 10.80 10.50 - 11.00 -	UT B	21 22	100/300 mm					-	Dark grey sandy GRAVEL. Sand is medium and coarse. Gravel is angular to subrounded fine and medium of limestone. [ESTUARINE DEPOSITS]	10.50	-10.17			
- 11.00 - 11.50 - -	В	23						11 -		(1.00				
- - 11.50 - 11.95 _ 11.50 - 11.95 - -	D SPT	24	N = 47 (S)					-	Stiff dark grey slightly sandy gravelly CLAY. Sand is fine to coarse. Gravel is angular to subrounded fine to coarse of of limestone psammite and quartzite. [ESTUARINE DEPOSITS]	11.50	-11.17			
- 12.00 - 12.50 - -	В	25						12 —						
- - 12.50 - 12.95 _ 12.50 - 12.95 - -	D SPT	26	N = 46 (S)					-						
- -13.00 - 13.50 - -	В	27						13 —		(3.50	)			
- 13.50 - 13.95 13.50 - 13.95	D SPT	28	N = 40 (S)					-						
—14.00 - 14.50 - - -	В	29						14 —						
- 14.50 - 14.74 14.50 - 14.74 -	D SPT	30	50/95 mm (S)					-	14.50m to 14.74m; dark grey slightly sandy clayey gravel. Sand is fine to coarse. Gravel is fine and medium of limestone.			* * * * * * * * * * * * * * * * * * * *		
- 15.00 - 15.50	В	31						_	Continued next page	15.00	-14.67			
-15.00 - 15.50 Notes	В	31						_	Continued next page	15.00	-14.67			

Print Date

-Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Client   Dublin Port Company		Locatio	n ID			
Fugro Reference		3FI	М	BH	-S	-04
Coordinates (m)   E718454.50 N734150.50   Ground Elevation (m Datum)   Cable Percussion to Rotary Coring		<b> ∵.</b>	'''—			•
Depth (m)   Type   No.   Test Results   TCR   SCR   ROD   Ft   Depth (%)   (	0.33	Sheet 4	4 of 6			
Type	-	Status		Pre	limina	ry
Type	ils	<u>'</u>		<u>'</u>		ındwater
Dark grey sandy GRAVEL with low co- content. Sand is fine to coarse. Grave is imestone psammite and quartzic.		Depth (Thickness (m)	s) Level (m Datun	m) Legend	Water Strike	
Very stiff dark grey singnty sandy graw with low cobbe content. Sand is fine to Gravel is angular to subrounded fine to filmestone psammite and quartzite. (<100mm x 80mm) are angular of dark limestone. [ESTUARINE DEPOSITS]	el is of obbles ents of	·				
The state of the s	to coarse. to coarse Cobbles k grey					
16.50 - 17.00 B 16.50 - 16.88 SPT  35 50/230 mm (S)  50/230 mm (S)	se. Gravel			7		
120mm) are subrounded of limestone [ESTUARINE DEPOSITS]  17.50 - 17.95   D   37   38   N = 35 (S)  Dark grey sandy GRAVEL with pocket dark grey clay and with low cobble co Sand is coarse. Gravel is subangular subrounded fine to coarse of limestone. [ESTUARINI DEPOSITS]  18.00 - 18.50   B   39   T	grey clay. s	16.50	-16.17	7		
17.50 - 18.00  B SPT  SPT  N = 35 (S)  N = 35 (S)  N = 35 (S)  N = 35 (S)  Dark grey sandy GRAVEL with pocket dark grey clay and with low cobble consand is coarse. Gravel is subangular subrounded fine to coarse of limeston Cobbles (-130mm x 110mm x 80mm) subangular of limestone. [ESTUARINI DEPOSITS]  Dark grey sandy GRAVEL with pocket grey stiff clay. Sand is coarse. Gravel subangular and subrounded fine to coarse. Gravel subangular and subrounded fine to coarse. Gravel subangular and subrounded fine to coarse. Gravel subangular and sandstone. [ESTUARINI DEPOSITS]  Stiff dark grey slightly sandy slightly gravely gr		(1.00)				
DEPOSITS]  Dark grey sandy GRAVEL with pocket grey stiff clay. Sand is coarse. Gravel subangular and subrounded fine to co limestone and sandstone. [ESTUARIN DEPOSITS]  18.50 - 18.72  D SPT SO/151 mm (S)  Stiff dark grey slightly sandy slightly ground is medium and coarse. Gargular to subrounded fine and medium and coarse. Gargular to subrounded fine to continue to the subrounded f	ntent. to ne. ) are	17.50		7		
18.50 - 18.72   SPT   50/151 mm (S)   Stirt dark grey slightly sandy slightly 9   CLAY. Sand is medium and coarse. G   angular to subrounded fine and mediu	ts of dark is parse of	18.00		7		
	ravel is	18.50	-18.17	7		
0 0 0 0 19 — 19 — 19 — 19 — 19 — 19 — 19		(1.17)				
19.50 - 19.67   D   41   50/100 mm (S)		19.67	-19.34	1		
				L		
Continued next page						

		Cont	ract Name	Dubl	in Po	rt MP	2 3F	-M		Locatio	n ID			
-fug	DA					rt Co				3FI	ЛБ	2Ц.	2	<b>Λ</b> /
			o Reference	F211		11 00	Праг	ıy		<b>31 I</b>	۷I_L	) I I.	<b>.</b>	V <del>4</del>
			dinates (m)	E718		50 N	73/11	50 50	Ground Elevation (m Datum) 0.33	Sheet 5	of 6			
			Type	_					ary Coring	Status	01 0	Preli	minar	·v
		•						1100		Otatus		1 1011		
Depth	Samp	ling an	d In Situ Testing			ecove			Strata Details	1	1	ı	Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
20.00 - 20.50				0	0	0				(1.33)				
- 20.50 - 20.86 20.50 - 20.86	D SPT	42	50/210 mm (S)	0	0	0		-						
- - 21.00 - 21.40	В	43						21 —	Dark grey slightly sandy clayey GRAVEL. Sand	21.00	-20.67	••••••		
21.00 - 21.50				80	0	0		-	is fine and medium. Gravel is subangular to rounded fine to coarse of limestone, rare psammite, rare quarts and rare calcite. [CLACIAL DEPOSITS] 21.20m to 21.50m; stiff dark gey slightly sandy slightly gravelly clay. Sand is fine and medium. Gravel is fine and medium of limestone, rare					
- 21.50 - 21.80 - 21.50 - 22.00	В	44		60	0	0		-	calcite and rare quartz. 21.40m to 21.50m; assumed zone of core loss.  21.80m to 22.00m; assumed zone of core loss.	(1.10)				
				20	0	0		22	Assumed zone of core loss.	22.10	-21.77	• • • • • •		
- 22.50 - 22.74 22.50 - 22.74  22.50 - 23.00	D SPT	45	50/90 mm (S)	0	0	0		-		(0.95)				
23.00 - 23.50	С	46	90 mm	90	18	0		23	Dark grey slightly clayey slightly sandy GRAVEL. Sand is medium. Gravel is subangular and subrounded medium and coarse of limestone sandstone and amphibolite [GLACIAL DEPOSITS]  Strong dark grey LIMESTONE with occasional	23.05 (0.36)	-22.72 -23.08			
23.54 - 23.77 23.41 - 23.90 23.50 - 24.25	С	47	130 mm	50	50	0	12		inclined (60-70°) calcite veins (<3mm). Fresh.  Discontinuities; Set#1, bedding plane fractures, inclined (0-10°), closely to medium spaced, planar and undulating, smooth, very tight, clean. [LIMESTONE BEDROCK] 23.41m to 23.90m; inclined (80°) calcite vein (<30mm). 23.90m to 24.25m; assumed zone of core loss.					
24.36 - 24.48	С	48	120 mm	100	96	47		-		(1.77)				
24.25 - 25.18 - - - Notes							6	-	Continued next page					

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Cont	tract Name	Dubl	in Po	rt MP	2_3F	М		Location	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	ıy		3FI	VI E	3H-	S-	04
		Fugr	o Reference	F211	210		-	•		• • •	··—			٠.
•		Cooi	rdinates (m)	E718	3454.	50 N	7341	50.50	Ground Elevation (m Datum) 0.33	Sheet 6	of 6			
		Hole	Туре	Cabl	e Per	cussi	ion to	Rota	ary Coring	Status		Preli	minar	у
Depth	Sampl	ing an	nd In Situ Testing	Co	re R	ecove	ery		Strata Details				Grour	idwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
25.18 - 25.27 25.27 - 25.68 25.68 - 25.75 25.00 - 26.50 25.75 - 25.83 25.83 - 26.03				100	74	40	10	- - - - -	Medium strong dark grey to black LIMESTONE Fresh locally distinctly weathered to destructured. Discontinuities; Set#1, bedding plane fractures, inclined (5-15°), very closely to medium spaced, planar, smooth, very tight, clean. [LIMESTONE BEDROCK] 25.18m to 25.27m; non-intact. Recovered as angular and subangular fine to coarse gravel-sized fragments. Possibly drilling disturbed distinctly weathered. 25.68m to 25.75m; non-intact. Recovered as angular and subangular fine to coarse gravel-sized		-24.85			
26.03 - 26.37 26.03 - 26.40	С	49	340 mm				4	26 — - - -	fragments. Possibly drilling disturbed distinctly weathered. 25.83m to 26.03m; distinctly weathered to destructured with angular and subangular fine and medium gravel-sized lithorelicts.  26.40m to 26.50m; assumed zone or core loss.					
26.50 - 26.86 26.90 - 27.40	В	50	50 mm	100			14	-	Strong dark grey LIMESTONE with inclined (70-90°) calcite veins (<20mm). Fresh.  Discontinuities; Set#1, bedding plane fractures, inclined (0-10°), very closely to closely spaced, planar and undulating, smooth and rough, tight, clean and with orangish brown staining. Set#2, inclinit		-26.17			
26.86 - 27.01 -26.50 - 27.40 27.01 - 27.40				100	51	0	13	27 —	joints, inclined (60-80°), planar, rough, tight and partly open with calcite mineralisation (<20mm) [LIMESTONE BEDROCK] 26.86m to 27.01m; non-intact. Recovered as subangular to subrounded medium and coarse gravel and cobble-sized fragments with calcite veins (<20mm).		-27.07			
27.40 - 27.67							15	-	Medium strong to strong dark grey to black locally grey LIMESTONE. Fresh locally moderately weathered. Discontinuities; Set#1, bedding plane fractures, inclined (0-10°), very closely spaced to medium spaced planar, smooth and rough, very tight, clean and with sil smear (<0.3mm) and clay smear (<0.5mm). Set#2, joints, inclined (40-50°), widely spaced,		-21.01			
28.07 - 28.21 27.40 - 28.90 27.67 - 28.80 28.31 - 28.63	С	52 51	140 mm 320 mm	93	84	67	7	28 —	alaman amanaka wamakintaka kinta alama	(1.50)				
-								- - 29 — - -	28.80m to 28.90m; assumed zone of core loss.  End of Borehole at 28.90 m	28.90	-28.57			
- Notes								- - - -						

Print Date

-Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Contract Name	!	Dublin Port MP2_3FM					Location	on ID	
fug	RO	Client		Dublin Port Company					3F	M BH	<b>1-S-04</b>
		Fugro Reference	се	F211210						_	
•		Coordinates (m	1)	E718454.50 N734150.50	Groun	d Elevation (m	n Datum) 0.3	33	Sheet	1 of 1	
		Hole Type		Cable Percussion and Rota	ry Corir	ng	·		Status	P	reliminary
				Standard Penet	tration T	est Results					
Test Depth (m)	Test Typ	oe Self Weight Penetration (mm)	Test	Result		Total Penetration (mm)	Hammer Serial Number	Energy R	atio (%)	Casing Depth (n	) Water Depth (m)
0.50	S	0	N=0	(0,0/0,0,0,0)		450	AR3214	54	1	0.50	
2.50	S	0	N=0	(0,0/0,0,0,0)		450	AR3214	54	1	2.50	
4 50	S	0	N=11	1 (0 3/4 5 1 1)		450	ΔR3214	5/	1	4 50	

			Standard Penetration I					
Test Depth (m)	Test Type	Self Weight Penetration (mm)	Test Result	Total Penetration (mm)	Hammer Serial Number	Energy Ratio (%)	Casing Depth (m)	Water Depth (m)
0.50	S	0	N=0 (0,0/0,0,0,0)	450	AR3214	54	0.50	
2.50	S	0	N=0 (0,0/0,0,0,0)	450	AR3214	54	2.50	
4.50	S	0	N=11 (0,3/4,5,1,1)	450	AR3214	54	4.50	
6.50	S	0	N=0 (0,0/0,0,0,0)	450	AR3214	54	6.50	
8.50	S	0	N=41 (6,9/11,13,9,8)	450	AR3214	54	8.50	
9.50	S	0	N=25 (4,3/4,6,7,8)	450	AR3214	54	9.50	
11.50	S	0	N=47 (4,6/8,11,12,16)	450	AR3214	54	11.50	
12.50	S	0	N=46 (3,5/8,10,10,18)	450	AR3214	54	12.50	
13.50	S S	0	N=40 (4,6/8,7,11,14)	450	AR3214	54	13.50	
14.50	S	0	N=50 (11,14/50 for 95mm)	245	AR3214	54	14.50	
15.50	S	0	N=47 (4,6/9,13,13,12)	450	AR3214	54	15.50	
16.50	S	0	N=50 (11,12/50 for 230mm)	380	AR3214	54	16.50	
17.50	S	0	N=35 (6,4/4,7,9,15)	450	AR3214	54	17.50	
18.50	S	0	N=50 (25 for 65mm/50 for 151mm)	216	AR3214	54	18.50	
19.50	S	0	N=50 (25 for 70mm/50 for 100mm)	170	AR3214	54	19.50	
20.50	S	0	N=50 (7,18/50 for 210mm)	360	AR3214	54	20.50	
22.50	S	0	N=50 (6,17/50 for 90mm)	240	AR3214	54	22.50	

	In Situ	Vane Test Res	ults	In Situ Hand Per	netrometer Results	Volatile Headspace Testir	g by Photoionisation Detector
Test Depth (m)	Test Type	Undisturbed Undrained Shear Strength (kPa)	Residual Undrained Shear Strength (kPa)	Test Depth (m)	Undisturbed Undrained Shear Strength (kPa)	Test Depth (m)	PID Result (ppm)

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL SPT Summary.hbt/Config Fugro Rev5/18/02/2019/TS

Print Date

				Con	tract Naı	me	Dublir	n Port MP	2_3FM									Locati	on IE	)		
-6	JGF	7		Clie	nt		Dublir	n Port Co	mpany									3F	М	R	H_9	<b>S-05</b>
	_ <del>_</del>	_ •			ro Refere		F2112		· · · y									JI	.41		(	J-00
					rdinates			315.00 N7	734103.9	90	Gro	ound	Elevation	(m Da	atun	n) 0.5	3	Sheet	1 of	1		
			-		Туре	` '		Percussi						,		, , , , , ,		Status			Prelim	ninary
											uipm											,
Depth From (m)	Depth To (	n) l	Hole Ty	ре	Date From	Date To	Е	Equipment	Core Ba	<u> </u>		e Bit	Drilling Crew	Logge	d By	Remarks						
0.00	17.42	1	CP	$\top$	13/11/2022	14/11/2022		Comacchio S1200, Pilcon					JW, SB, CR, AB	OD, F								
17.00	29.40		RC		14/11/2022	14/11/2022		Wayfarer cchio MCS120	00				JW, SB, CR,	OD, F								
	20.40				,_022		Some						AB AB	PE	) <sup>-</sup> ,							
					Progr								Ro	otary [							ore De	etails
Date (dd/mm/yyyy	) (hh:	ne mm)		le Dep (m)	(m)	epth Water Dept (m)	vveau	ier		Dep From	n (m)	epth To (m)	Flush Ty	ре	(	70)	lush Colou	(nn.n	nm) f	Depth From (m)	Depth To (m)	Diameter (mm)
13/11/2022 13/11/2022	2 15 2 18	:30 :30		0.00 7.50	0.00 7.50		Good Windy	, Chilly		17. 17.	00	17.50 18.00	W W		1 1	00 00	Grey Grey	00:0 00:0	03 04	17.00 17.50	17.50 18.00	102 102
13/11/2022 14/11/2022	2 19 2 05	:00		7.50 16.30	7.50		'	•		18. 18.	00	18.50 19.00	W W		1	00	Grey Grey	00:0	04 05	18.00 18.50	18.50 19.00	102 102
14/11/2022 14/11/2022	2 07	45		16.00 17.42	16.00 17.42					19. 19.	00	19.50 20.00	w w		1	00 00	Grey Grey	00:0	05	19.00 19.50	19.50 20.00	102 102
14/11/2022	2 18	:30		26.00 26.00	17.00					20. 20.	00	20.50 21.00	w w		1	00	Grey Grey	00:0	05	20.00	20.50 21.00	102 102
14/11/2022		24		29.40						21.	00	21.50 21.50 22.00	w w		1	00	Grey Grey	00:0	05	21.00 21.50	21.50 22.00	102 102 102
										22. 22.	00	22.50 23.00	w w		1	00 00	Grey Grey	00:0	06	22.00 22.50	22.50 23.00	102 102 102
										23. 23.	00	23.50 24.00	W W		1	00 00	Grey Grey	00:0	07	23.00 23.50	23.50 23.50 24.00	102 102 102
										24.	00	24.50	W W		1	00 00 00	Grey	00:0	06	24.00	24.50	102
										24. 25.	00	25.00 25.50	W		1	00	Grey Grey	00:0	06	24.50 25.00	25.00 25.50	102 102
										25. 26.	00	26.00 26.50	W W		1	00	Grey	00:0	06	25.50 26.00	26.00 26.50	102 102
										26. 26.	80	26.80 28.10	W W		1	00	Grey Grey	00:0	16	26.50 26.80	26.80 28.10	102 102
										28.	10	29.40	w		1	00	Grey	00:	ıb	28.10	29.40	102
				Н	lole and	Casing																
Depth		$\perp$	Hole [		ter (mm)	Depth To	(m)	Casing Diar														
7.5 16.	.30			200 200		0.00 15.00		20	10													
17. 26.	.00			200 146		17.00		20	IU													
29.	.40			146																		
			CI	nisel	lling / Slo	ow Progre	ss	1		1												
Depth F		$\perp$	De	pth To		Duration (hh	n:mm)	Tool / R	temark	1												
10. 15.	.40			11.00 16.00	0	00:48 02:00																
16.	.00			17.00	U	02:15																
			1		21.71			101		-												
	s	,1 -	Wat Time Ela		Strike	T		Water A	Added Depth To	-												
Strike At (m)	Rise To (I	n)	(min	s)	Casing Dept	tn (m) Depth S	ealed (m)	(m)	(m)	-												
			Mata	r Q+	ike Rem	narke				<u> </u>				C	ner	al Rem	narke					
Groundwate	er observa					en ro		Bo	rehole adva	nced	using c	able pe	rcussion with s					testing (S	SPT). C	lient inst	ucted rota	ary switch at
													at 29.40m metr					3 (*	,. 5			,
			Inst	allat	tion						Pipe				Т			F	Backt	fill		
Туре	Tip Dept	h /	Respo	nse Zo		Zone Installation	on Date	ID To	op Depth (m)		Depth (r	$\neg$	meter (mm)	Туре	De	pth From (	m) Depth 1			ckfill Mat	erial	Date
	DISIGNICE	(111)	10	(۱۱۱) ب	Dase (	,								-	$\top$		† ·					
Notes					-			-														
- Abbrevi	ations a	ınd	resul	ts da	ata define	d in 'Explor	atory L	_ocation R	ecords K	eysh	eets'											
Checked By	,						E	Elevation Datu	m	Du	ublin Po	ort Char	t Datum		Gr	id Coordi	nate Syster	m  17	ГМ			
Template: F	GSL/HBS	/FGS	SL BH	Summ	nary.hbt/Conf	fig Fugro Rev5	/26/06/20	019/TS+AW										Print Dat	е		17/11/202	22

<b>Tug</b> i	RO	Clier					2_3F							
			nt	Dubl	in Po	rt Co	mpar	าง		3FI	Л Б	RH.	s.	.05
•			o Reference	F211						01 1	··—	<b>-</b>		00
				E718	3615.	00 N	7341	03.90	Ground Elevation (m Datum) 0.53	Sheet 1	of 6			
		Hole	Туре	Cabl	e Per	cussi	ion to	Rota	ary Coring	Status		Preli	minaı	у
Depth	Sampl	ing an	d In Situ Testing	Co	re Re	ecove	ery		Strata Details				Groui	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
0.00 - 0.50	В	1						-	Dark grey slightly clayey sandy GRAVEL with rare shell fragments (<20mm x 12mm) and strong hydrocarbon odour. Sand is fine and medium. Gravel is rounded medium and coarse of sandstone limestone and mudstone. [POSSIBLE MADE GROUND]	(0.50)				
- 0.50 - 0.95 0.50 - 1.00 0.50 - 0.95	D B SPT	2 3	N = 10 (S)					-	Loose dark grey and grey sandy GRAVEL with strong hydrocarbon odour. Sand is fine to coarse. Gravel is subangular to rounded fine and medium of limestone sandstone quartz calcite and siltstone. [POSSIBLE MADE GROUND]	(0.50)	0.03			
- 1.00 - 1.50 -	В	4						1	Grey gravelly SAND with frequent shells (<45mm x 30mm) and frequent shell fragments (<25mm x 20mm). Sand is fine and medium.	1.00	-0.47			
- - 1.50 - 1.95	D	5						-	Gravel is angular to subrounded fine and medium of mixed lithologies including sandstone limestone quartz and siltstone. [POSSIBLE MADE GROUND]	(0.50)	-0.97			
1.50 - 1.95 1.50 - 2.00 1.50 - 1.95	B SPT	6	N = 5 (S)					-	Loose grey slightly silty SAND with frequent shells (<30mm x 20mm) and frequent shell fragments (<20mm x 10mm). Sand is fine and medium. [POSSIBLE MADE GROUND]	(0.50)	-0.97			
- - 2.00 - 2.50	В	7						2 —	Soft greyish brown slightly gravelly sandy CLAY	2.00	-1.47			
- - -								-	with some shell fragments (<30mm x 20mm) and moderate sulphurous odour. Sand is fine and medium. Gravel is subangular and subrounded fine and medium of limestone and sandstone. [ESTUARINE DEPOSITS]	(0.50)				
- 2.50 - 3.50 2.50 - 3.50	B PS	9 8						-	Soft greyish brown sandy silty CLAY with strong sulphurous odour. Sand is fine. [ESTUARINE DEPOSITS]	2.50	-1.97			
- - - -								3		(1.00)		X X X X X X X X X X X X X X X X X X X		
- 3.50 - 3.95 3.50 - 3.95	D SPT	10	0/450 (S)					-	Soft greyish brown slightly sandy silty CLAY with rare shell fragments (<5mm x 3mm) and strong sulphurous odour. Sand is fine. [ESTUARINE DEPOSITS]	3.50	-2.97	X X X X X X X X X X X X X X X X X X X		
- 4.00 - 4.50	В	11						4				X X X X		
- 4.50 - 5.50 4.50 - 5.50	B PS	13 12						-		(2.50)		X X X X X X		
_ Notes								_	Continued next page			×		

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Con	tract Name	Dubl	lin Po	rt MP	2 3F	-M		Loca	tion	ID			
-fire	RO	Clier	nt			rt Co				<b>3</b> F	= <b>N</b> /	1 6	3H-	<b>S</b> _	<b>05</b>
			o Reference	F211		11 00	Пра	ıy		JI	IV	' <u>-</u> '	) I I.	<b>.</b>	03
			rdinates (m)			00 N	7341	03.90	Ground Elevation (m Datum) 0.53	Shee	t 2 c	of 6			
			туре						ary Coring	Statu			Preli	minar	y
	Samp	'	d In Situ Testing	Co	ore R	ecove	ery		Strata Details	·			<u> </u>		ndwater
Depth (m)			Test		SCR		FI	Depth	Charles December 1	Dep	oth	Level	l	Water	Backfill /
	Туре	No.	Results	(%)		(%)	(-)	(m)	Strata Descriptions	(Thick	ness) (n	n Datum)	Legend	Strike	Installation
-								-					× ×		
								-					×		
- 5.50 - 5.95	D	14						_							
5.50 - 5.95	SPT		0/450 (S)										×		
								-					×		
-								-					×		
ŧ								-					×		
- 6.00 - 6.50 6.00 - 6.50	B D	16 15						6 —	Soft greyish brown silty CLAY. [ESTUARINE	6.0	10	-5.47	× ×		
- 0.00								-	DEPOSITS]				×		
-								-		(0.5	50)		×		
								-					×		
- 6.50 - 7.50	PS	17								6.5	50	-5.97	×		
- 0.00 7.00									Very soft and soft, locally firm, greyish brown slightly sandy silty CLAY with some shell	0.0		0.07	<u>×_×</u> _		
									fragments (<3mm x 50mm). Sand is fine.				<u>×</u> _×		
-								-	[ESTUARINE DEPOSITS].				<u>×</u> ×		
-								-							
_								7 —					× ×		
÷								-					× ×		
İ								-					×		
													×		
- 7.50 - 8.00	В	19											×		
7.50 - 8.00	D	18											×		
-								-		(0.5	-0)		×		
-								-		(2.5	,0)		×		
-								-					×		
- 8.00 - 8.45 8.00 - 8.45	D SPT	20	0/450 (S)					8 —	8.00m to 8.45m; firm locally fissured.				×		
-								-					<u>×_×</u> _		
													×		
													×_×_		
- 8.50 - 9.00	В	21						_					<u>×_×</u> _		
8.50 - 9.00	D	22						-					<u>×</u> _×		
-								-					<u>×</u> ×		
-								-					××		
ŀ								-							
- 9.00 - 9.20 9.00 - 9.50	PS B	23 24						9 —	Very soft brownish grey gravelly silty CLAY with	9.0	10	-8.47	<u> </u>		
									low cobble content. Gravel is subangular and angular medium to coarse of limestone.				<u></u>		
									Cobbles (<60mm x 80mm x 110mm) are subangular of dark grey limestone.	(0.5	i0)		×—		
									[ESTUARINE DEPOSITS].				×		
- 9.50 - 10.00	В	25						-	Dense dark brown and grey silty sandy	9.5	50	-8.97	×		
								-	GRAVEL with low to medium cobble content,				× × ×		
}								-	becoming sandy gravelly CLAY with depth. Sand is fine to coarse. Gravel is subangular				× × ×		
-								-	and subrounded, elongated, fine to coarse of				×××		
·	_							-	limestone, siltstone and sandstones. Cobbles (<30mm x 80mm x 80mm) are of limestone.				××		
-10.00 - 10.45 10.00 - 10.45	D SPT	26	N = 31 (S)					-	Continued next page				X		
Notes	•		• • • •		•										

		Con	tract Name	Dubl	in Po	rt MP	2_3F	-M		Locatio	n ID			
-fug	RO	Clie	nt	Dubl	in Po	rt Coı	mpar	าy		3FI	<b>N</b>	BH.	S-	05
		_	ro Reference	F211										
·			rdinates (m)	_				03.90	, ,	Sheet 3	of 6	Desti	minar	
			е Туре					Rola	ary Coring	Status		Preli	minar	у
Depth	Samp	ling ar	nd In Situ Testing	Co	re R	ecove	ery		Strata Details				Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum	Legend	Water Strike	Backfill / Installation
								-	[ESTUARINE DEPOSITS].	(1.50)		* * * * * * * * * * * * * * * * * * *		
- 10.50 - 11.00 10.50 - 11.00	B D	27 28						-	10.70m to 11.00m; sandy gravelly clay					
11.00 - 11.45 11.00 - 11.45	D SPT	29	N = 39 (S)					11 —	Very stiff to stiff dark brownish grey fissured slightly sandy silty CLAY. Sand is fine to coarse predominately fine. [PORT CLAY].  11.00m to 11.45m; slightly gravelly. Gravel is subangular and rounded fine of limestone.	11.00	-10.47	× × × × × × × × × × × × × × × × × × ×		
- 11.50 - 12.00 11.50 - 12.00	B D	30 31						-		(1.50)		x x x x		
-12.00 - 12.45 12.00 - 12.45	D SPT	32	N = 21 (S)					12	12.00m to 12.45m; firm to stiff.			× × × × × × × × × × × × × × × × × × ×		
- 12.50 - 13.00 - -	В	33						-	Soft and firm gravelly CLAY. Gravel is subangular to subrounded fine to coarse of limestone. [PORT CLAY].	12.50	-11.97	* * * * * * * * * * * * * * * * * * * *		
- 13.00 - 13.50 - -	UT	34	92/500 mm					13 —	Very stiff dark brown slightly sandy gravelly CLAY. Sand is fine to coarse. Gravel is subangular to rounded, fine to coarse of dark grey limestone. [PORT CLAY].	13.00	-12.47			
- - 13.50 - 14.00 - -	В	35						-						
-14.00 - 14.45 14.00 - 14.45 -	D SPT	36	N = 28 (S)					14 —				* * * * * * * * * * * * * * * * * * *		
- 14.50 - 15.00 - - - - - -15.00 - 15.42	B	37						-		(3.00)		* · · · · · · · · · · · · · · · · · · ·		
15.00 - 15.42 15.00 - 15.42 Notes	SPT	JU	50/265 mm (S)						Continued next page					

		Conf	tract Name	Dubl	in Po	rt MP	2 3F	FM		Locat	on ID			
-fire	RO	Clier	nt			rt Co				3F	М	BH.		.05
			o Reference	F211		11 00	пра	ıy		JI	1VI —	DI I	· <b>J</b> ·	-03
		_	rdinates (m)	_		UU N.	73/1	03.90	Ground Elevation (m Datum) 0.53	Sheet	4 of 6			
			Type	_					ary Coring	Status		Preli	mina	rv
		11010	Турс					1 100	ary coming	Otata		1 101	I	y
Depth	Samp	ling an	nd In Situ Testing		1	ecove			Strata Details			1	Grou	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	(-)	Depth (m)	Strata Descriptions	Dept (Thickn (m)	Level (m Datur	Legend	Water Strike	Backfill / Installation
- - - - - - - - - - - - - - - - - - -	В	39						- - - - - -		16.0	0 -15.47			
16.00 - 16.50 16.00 - 16.30 - - - - - 16.50 - 17.00	B SPT	41	50/190 mm (S)					-	Very dense brown silty gravelly SAND with pockets (<90mm x 50mm x 30mm) of silty clay Sand is medium and coarse. Gravel is angular to subrounded fine to coarse of limestone. [GLACIAL DEPOSITS]			* * * * * * * * * * * * * * * * * * *		
- - - -17.00 - 17.42	D	43							Assumed zone of core loss.	17.0	0 -16.47			
17.00 - 17.42	SPT		50/270 mm (S)					-	Assumed zone of core loss.					
17.00 - 17.50				0	0	0				(0.50	0)			
17.50 - 18.00 17.80 - 17.90	D	44	100 mm	80	0	0		-	Very dense dark brown slightly sandy very clayey GRAVEL. Sand is medium and coarse. Gravel is subangular and subrounded fine to coarse of limestone. [GLACIAL DEPOSITS]	(0.4)	0)			
-18.00 - 18.29	D	45						18	Assumed zone of core loss.					
18.00 - 18.29 - 18.00 - 18.50	SPT		50/140 mm (S)	0	0	0		-		(0.6)	0)			
- - 18.70 - 18.80 - 18.50 - 19.00	D	46		82	0	0		-	Very dense dark brown slightly sandy clayey GRAVEL with low cobble content. Sand is medium and coarse. Gravel is subangular and subrounded fine to coarse of limestone. Cobbles (<90mm x 75mm x 65mm) are subangular of limestone. [GLACIAL	(0.4	1)			
-19.00 - 19.42 19.00 - 19.42	D SPT	47	50/270 mm (S)					19 —	DEPOSITS] Assumed zone of core loss.	18.9	1 -18.38	3		
19.00 - 19.50				0	0	0		-		(0.59	9)			
- - 19.70 - 19.80 - 19.50 - 20.00	D	48		100	0	0		-	Dark brownish grey slightly sandy very clayey GRAVEL with low cobble content. Sand is medium and coarse. Gravel is subangular and subrounded fine to coarse of limestone. Cobbles (<100mm x 75mm x 50mm) are subrounded of limestone. [GLACIAL DEPOSITS]	(0.5)				
-20.00 - 20.45	D	49	N 00 (0)	-				-	Continued next page	20.0	0 -19.47	, <del>  * * * * *</del>	1	
20.00 - 20.45	SPT		N = 39 (S)						Johnman Horr page					

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Con	tract Name	Dubl	in Po	rt MP	<sup>2</sup> _3F	-M		Location	n ID			
-fug	RO	Clie	nt	Dubl	in Po	rt Co	mpar	าง		3FI	и і	RH.	S.	.05
			ro Reference	F211				-,		0	··—	<b>J</b>	J	
•		Coo	rdinates (m)	E718	3615.	00 N	7341	03.90	Ground Elevation (m Datum) 0.53	Sheet 5	of 6			
		Hole	Туре	Cabl	e Pei	cuss	ion to	Rot	ary Coring	Status		Preli	minaı	У
Depth	Samp	ling ar	nd In Situ Testing	Co	re R	ecove	ery		Strata Details		,	_	Grou	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
									Assumed zone of core loss.					
20.00 - 20.50				0	0	0				(0.50)				
								-	Dark brownish grey slightly sandy clayey GRAVEL. Sand is medium and coarse. Gravel is subangular to rounded fine to coarse of	20.50	-19.97	•		
20.70 - 20.83 20.50 - 21.00	D	50		66	0	0			limestone. [GLACIAL DEPOSITS]  Assumed zone of core loss.	20.83	-20.30	-		
- -21.00 - 21.36 21.00 - 21.36	D SPT	51	50/210 mm (S)					21 -	, resulting 25/10 of color local.					
21.00 - 21.50				0	0	0				(0.67)				
- - 21.60 - 21.70	D	52	100 mm					-	Dark brownish grey slightly sandy clayey GRAVEL. Sand is medium and coarse. Gravel is subangular to rounded fine to coarse of	21.50	-20.97	•		
21.50 - 22.00				100	0	0			limestone. [GLACIAL DEPOSITS]	(0.50)				
-22.00 - 22.45 22.00 - 22.45	D SPT	53	N = 40 (S)					22 -	Assumed zone of core loss.	22.00	-21.47	· · · · · ·		
22.00 - 22.50				0	0	0				(0.50)				
- - 22.60 - 22.70	D	54						-	Dark brownish grey slightly sandy clayey GRAVEL. Sand is medium and coarse. Gravel	22.50	-21.97	****		
22.50 - 23.00				80	0	0			is subangular to rounded fine to coarse of limestone and sandstone. [GLACIAL DEPOSITS]	(0.40)				
- -23.00 - 23.38 23.00 - 23.38	D SPT	55	50/230 mm (S)					23 -	Assumed zone of core loss.	22.90	-22.37	** · <u>*</u>		
23.00 - 23.50				0	0	0				(0.60)				
- - 23.60 - 23.70	D	56						-	Very dense dark brownish grey slightly clayey slightly sandy GRAVEL. Sand is medium.	23.50	-22.97			
23.50 - 24.00				60	0	0			Gravel is angular to subrounded medium and coarse of limestone. [GLACIAL DEPOSITS]  Assumed zone of core loss.	23.80	-23.27			
-24.00 - 24.40 24.00 - 24.40	D SPT	57	50/250 mm (S)					24 —		(0.70)				
24.00 - 24.50				0	0	0								
- 24.50 - 24.60 24.50 - 25.00	D	58		54	0	0		-	Very dense dark grey slightly clayey slightly sandy GRAVEL. Sand is fine. Gravel is subangular coarse of limestone. [GLACIAL DEPOSITS]	24.50 (0.27)	-23.97 -24.24			
- - -25.00 - 25.39	D	59							Assumed zone of core loss.					
25.00 - 25.39	SPT		50/240 mm (S)						Continued next page					

		Con	tract Name	Dubl	in Po	rt MP	2_3F	-M		Location	טו ו			
-fug	RO	Clier	nt	Dubl	in Po	rt Co	mpar	ny		3FN	ЛF	BH.	S.	05
			ro Reference	F211						<b>0.</b> .	··—	<b>-</b>		00
•			rdinates (m)			00 N	7341	03.90	Ground Elevation (m Datum) 0.53	Sheet 6	of 6			
			: Type						ary Coring	Status		Preli	mina	гy
	Samp		nd In Situ Testing			ecove			Strata Details					ndwater
Depth (m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill /
			rtocuito	()	()	,	( )							
25.00 - 25.50 -				0	0	0				(1.23)				
25.50 - 26.00				0	0	0								
-26.00 - 26.38 26.00 - 26.38 26.00 - 26.50	D SPT	60	50/226 mm (S)	100	0	0		26 -	Very dense dark grey slightly clayey slightly sandy GRAVEL with low cobble content. Sand is fine to coarse. Gravel is angular to to rounded fine to coarse of limestone, sandstone and schist. Cobbles (<40mm x 60mm x 140mm) are subangular and subrounded of	26.00	-25.47			
- 26.50 - 26.80	В	61						-	limestone and green schist. [GLACIAL DEPOSITS]					
26.50 - 26.80				100	0	0								
26.80 - 27.09 							14	27 –	Medium strong to strong dark grey LIMESTONE locally with some crinoids fragments (<2mm). Fresh locally slightly weathered. Discontinuities; Set#1, bedding plane fractures, inclined (20-30°), very closely to closely spaced, planar, undulating and	26.80	-26.27			
26.80 - 28.10 - 27.20 - 28.10				100	86	55	4	-	stepped, smooth and rough, very tight to partially open, clean locally infilled with clay (<0.5mm), and occasional to frequent clay smears (<0.3mm). [LIMESTONE BEDROCK] 27.09m to 27.20m; distinctly weathered with very closely spaced bedding plane fractures to non-intact recovered as angular fine to medium gravelsized fragments.	(1.30)				
- 27.87 - 28.08 -	С	62	210 mm					28 -	27.87m to 28.10m; some crinoids fragments (<2mm).	28.10	-27.57			
- - -									Strong dark grey mottled grey LIMESTONE with abundant crinoids fragments (<2mm) some shell fragments (<2mm x 10mm) occasional possible grey burrow (<30mm x 45mm) and occasional calcite veins (<2mm) and rare incipient fractures. Fresh. Discontinuities;		-27.37			
28.10 - 29.06 28.73 - 28.99 28.10 - 29.40	С	63	260 mm	100	79	54	6	29 —	Set#1, bedding plane fractures, inclined (0 to 20°), very closely to medium spaced, planar, rough, very tight, clean. Set#2; joints, inclined (50-60°), widely spaced, planar, smooth and rough, very tight, clean. [LIMESTONE BEDROCK]  28.46m to 28.62m; some and frequent stylolites.	(1.30)				
- 29.06 - 29.13 - 29.13 - 29.24 - 29.24 - 29.40							9		29.06m to 29.13m; non-intact. Recovered as angular and subangular fine to coarse gravel-sized fragments. 29.12m to 29.26m; inclined (80-90°) bifurcating calcite vein (<3mm). 29.24m to 29.40m; non-intact. Recovered as angular and subangular fine to coarse gravel-sized	29.40	-28.87			
-									fragments. End of Borehole at 29.40 m					

Location ID Contract Name Dublin Port MP2\_3FM 3FM\_BH-S-05 Client Dublin Port Company Fugro Reference F211210 Coordinates (m) E718615.00 N734103.90 Ground Elevation (m Datum) 0.53 Sheet 1 of 1 Hole Type Cable Percussion and Rotary Coring Status

			Standard Penetration T	oot Dooulto				
	Call	f Weight		Total Penetration	Hammer Serial			
	Type Penetr	ration (mm)	Test Result	(mm)	Number	Energy Ratio (%)	Casing Depth (m)	Water Depth (m)
0.50			N=10 (3,5/3,3,2,2)	450	AR3214	54	0.50	0.00
1.50	3		N=5 (2,3/1,2,2,0)	450	AR3214	54	1.50	0.00
3.50	3		N=0 (0,0/0,0,0,0)	450	AR3214	54	3.50	0.00
1.50 S 3.50 S 5.50 S 8.00 S	3		N=0 (0,0/0,0,0,0)	450	AR3214	54	5.50	0.00
8.00	3   1		N=0 (0,0/0,0,0,0)	450	AR3214	54	8.00	0.00
10.00	<u> </u>		N=31 (8,8/6,7,10,8)	450	AR3214	54	10.00	0.00
11.00	2		N=39 (3,5/6,10,9,14)	450 450	AR3214	54	11.00	0.00
12.00	2		N=21 (3,3/4,5,5,7)	450 450	AR3214	54 54	12.00	0.00
14.00 S 15.00 S	2		N=28 (5,6/5,6,8,9) N=50 (7,11/50 for 265mm)	450 415	AR3214 AR3214	54 54	14.00 15.00	0.00 0.00
16.00			N=50 (7,11/50 for 20511111) N=50 (25 for 110mm/50 for 190mm)	300	AR3214 AR3214	54 54	16.00	0.00
17.00	3		N=50 (23 101 11011111/30 101 19011111)	420	AAR3214	54 54	17.00	0.00
18.00			N=50 (7,9/50 for 140mm)	290	AR3214	54	18.00	0.00
19.00			N=50 (5,9/50 for 270mm)	420	AR3214	54	19.00	0.00
20.00	š		N=39 (3,4/4,9,10,16)	450	AR3214	54	20.00	0.00
21.00	š		N=50 (6,12/50 for 210mm)	360	AR3214	54	21.00	0.00
22.00	5		N=40 (6,8/5,7,9,19)	450	AR3214	54	22.00	0.00
23.00	3		N=50 (5,5/50 for 230mm)	380	AR3214	54	23.00	0.00
10.00 11.00 12.00 14.00 15.00 16.00 17.00 18.00 19.00 20.00 21.00 22.00 23.00 24.00 25.00	s	0	N=50 (5,8/50 for 250mm)	400	AR3214	54	24.00	0.00
25.00	3	0	N=50 (4,15/50 for 240mm)	390	AR3214	54	25.00	0.00
26.00	3	0	N=50 (6,7/50 for 226mm)	376	AR3214	54	26.00	0.00

	In Situ	Vane Test Resi	ulte	In Situ Hand Per	netrometer Results	Volatile Headspace Testing by Photoionisation Detector			
						· · · · · · · · · · · · · · · · · · ·			
Test Depth (m)	Test Type	Undisturbed Undrained Shear Strength (kPa)	Residual Undrained Shear Strength (kPa)	Test Depth (m)	Undisturbed Undrained Shear Strength (kPa)	Test Depth (m)	PID Result (ppm)		
1									

### Notes

Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL SPT Summary.hbt/Config Fugro Rev5/18/02/2019/TS

Print Date

17/11/2022

Preliminary

			Coi	ntract Na	ame	Dubli	in Port MF	P2_3FM								Locatio	on ID		
-6	JGR		Clie	ent		Dubli	in Port Co	mpany								3FI	М	RH-	S-06
				gro Refe	rence	F211										<b>J</b> I 1	' <b>''</b>	- ۱۱ ا	0-00
•			_	ordinate			621.80 N	734133	30	Gro	und	Elevation	(m D	atum)  -	1.05	Sheet	1 of 1		
			_	е Туре	` /		e Percuss			_			,			Status	-	Prelin	ninary
				7.		1			_	uipme								<u> </u>	•
Depth From (m)	Depth To (m	Hole	Туре	Date From	Date To		Equipment	Core Ba	·	Core		Drilling Crew	Logge	d By Remar	ks				
0.00	17.00	С	P	12/11/2022	13/11/20		Comacchio S1200, Pilcon					JW, SB	FS, C	DD,					
17.00	28.80	R	c	13/11/2022	13/11/20		Wayfarer acchio MCS12					CR, SB	PD/FS						
						_						,							
				Prog	ress			•				Ro	tary I	Details				Core D	etails
Date (dd/mm/yyyy	Tim (hh:m	e m)	Hole De	epth Casing I	Depth Water De (m)	<sup>pth</sup> Weat	her		Dep From	oth De	pth To (m)	Flush Typ	ре	Flush Return (%)	Flush Colou	ır Run Tir (hh:mn	me Dep	th Depth To (m) (m)	Diameter (mm)
12/11/2022 12/11/2022	08:0		0.00	0.0	)	Dry V Dry V			17. 17.		7.50 9.00	W W		80 100	Grey Grey	00:08 00:13		0 17.50	102 102
12/11/2022 13/11/2022	19:1	6	12.5 17.0	0   12.5	0		•		19. 20.	00 20	0.50 2.00	W W		100 100	Grey Grey	00:11 00:14	1 19.0	0 20.50	102 102
13/11/2022	05:1		28.8	0 17.0					22. 22.	00 2	2.75 3.50	W		100 100	Grey Grey	00:06	6 22.0	0 22.75	102 102
									23.	50 24	4.25 5.00	w		100	Grey Grey	00:06	6 23.5	0 24.25	102 102
									25. 26.	00 20	6.50 8.00	w		100 100 100	Black Black	00:14	4 25.0	0 26.50	102 102 102
									28.		8.80	W		100	Black	00:10			102
	-			Hole and	d Casing				1										
Depth	To (m)	Hole		eter (mm)	Depth 1	o (m)	Casing Dia	meter (mm)	1										
12. 17.			20 20		8.0 16.3			00											
28.			14		17.0			00											
		(	Chise	elling / S	low Prog	ess													
Depth F	rom (m)	1	Depth '		Duration (		Tool / F	Remark											
8.0 15.	00 20		8.3 15.	30 50	01:0 01:0	10													
16.			16.		00:														
		W	ater	Strike			Water	Added											
Strike At (m)	Rise To (m	) Time (r	Elapseo	Casing De	pth (m) Depti	Sealed (m	Depth From (m)	Depth To (m)											
				trike Re										neral Re					
Groundwate	er observati	on not r	ecorde	d over water								rcussion with sat 28.80m metr				testing (SF	PT). Client	instructed ro	tary switch at
						1								_					
	T- 0		stalla	ation Zone Respon	no Zor - I				1	Pipe		ı					ackfill		_
Туре	Tip Depth Distance (	n) Res	Top (m)	) Base	se Zone (m) Install	ation Date	ID 1	Top Depth (m)	Base	Depth (m	) Dian	neter (mm)	Туре	Depth From	n (m) Depth	To (m)	Backfill	Material	Date
N																			
Notes	_4:_		w ·	-4		•	=												
- Abbrevi	ations ar	nd res	ults d	ata defin	ed in 'Expl	oratory	Location R	ecords K	eysh	eets'									
		-				1			_					T_		П			
Checked By							Elevation Datu	ım	Du	ublin Por	t Char	Datum		Grid Coo	rdinate Syste				
Template: F	GSL/HBSI/	GSL B	H Sum	mary.hbt/Co	nfig Fugro Re	v5/26/06/2	2019/TS+AW									Print Date		16/11/20	22

		Conf	tract Name	Dubl	in Po	rt MP	2_3F	-M		Locat	on ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Coi	mpar	าง		3F	<b>M</b> _	RH.	S.	.06
			o Reference	F211				-,		<b> </b>	'''-	<b>D</b> 111	J	UU
•			rdinates (m)	_		80 N	7341	33.30	Ground Elevation (m Datum) -1.05	Sheet	1 of 6			
			Туре	_					ary Coring	Status		Preli	mina	ry
Depth	Samp	ling an	nd In Situ Testing	Co	ore R	ecove	ery		Strata Details				Grou	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Dept (Thicknown) (m)	Level (m Datum	Legend	Water Strike	Backfill / Installation
0.00 - 0.50	В	1						-	Very soft black slightly sandy silty CLAY with strong sulphurous odour. Sand is fine. [RECENT ESTUARINE DEPOSITS].			X X X X		
- 0.50 - 0.95 0.50 - 1.50 0.50 - 0.95	D B SPT	2 3	0/450 (S)					1-				X X X X X X X X X X X X X X X X X X X		
- 1.50 - 2.50 1.50 - 2.50	B PS	5 4						2 —		(3.50	)	X X X X X X X X X X X X X X X X X X X		
- 2.50 - 2.95 2.50 - 3.50 2.50 - 2.95	D B SPT	6 7	0/450 (S)					3 -				X		
- 3.50 - 4.50 3.50 - 4.50	B PS D B	9 8 10 11						4-	Soft black sandy silty CLAY with frequent plant rootlets (<50mm x 2mm) and moderate hydrocarbon odour. Sand is fine. [RECENT ESTUARINE DEPOSITS]	3.50	-4.55	X X X X X X X X X X X X X X X X X X X		
4.50 - 4.95	SPT	•••	0/450 (S)					-				^XXXXXXXXXX		
										(3.00				

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Con	tract Name	Dubl	in Po	rt MP	2_3F	-M		Location	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Coi	— mpar	าง		3F	<b>M</b> _I	RH.	s.	.06
			ro Reference	F211				-,		<b> </b>	'''—'	<b>-</b> 111	J	UU
•			rdinates (m)	E718	3621.	80 N	7341	33.30	Ground Elevation (m Datum) -1.05	Sheet	2 of 6			
		Hole	Туре	Cabl	e Per	cussi	ion to	Rota	ary Coring	Status		Preli	minaı	ry
Depth	Samp	ling an	nd In Situ Testing	Co	ore R	ecove	ery		Strata Details				Groui	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thicknes (m)	s) Level (m Datum	Legend	Water Strike	Backfill / Installation
- 5.50 - 6.50 5.50 - 6.50	B PS	13 12							5.50m to 6.50m; slightly sandy.			X- X		
- 6.50 - 6.95 6.50 - 7.50 6.50 - 6.95	D B SPT	14 15	0/450 (S)					-	Soft black sandy silty CLAY with rare pockets (<30mm x 25mm) of soft brown clay. Sand is fine and medium. [RECENT ESTUARINE DEPOSITS]	(0.50) (0.50)	-7.55 -8.05	X X X X X X X X X X X X X X X X X X X		
- 7.50 - 7.95 7.50 - 8.00 7.50 - 7.95	D B SPT	16 17	N = 46 (S)					-	Dense dark grey slightly clayey sandy GRAVE with low cobble content. Sand is medium and coarse. Gravel is angular to subrounded fine to coarse of limestone. Cobbles (<120mm x 75mm x 30mm) are subangular of limestone sandstone and calcite. [ESTUARINE DEPOSITS]	L.				
- 8.00 - 8.50	В	18						8 —	Dense grey sandy GRAVEL with medium cobble content. Sand is medium and coarse. Gravel is subangular to rounded fine to coarse of limestone, sandstone, quartz, calcite and	8.00	-9.05	o 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		
- 8.50 - 8.95 8.50 - 9.00 8.50 - 8.95	D B SPT	19 20	N = 35 (S)					-	green schist. Cobbles (<110mm x 110mm x 50mm) are subangular and subrounded fine to coarse of limestone and sandstone.  [ESTUARINE DEPOSITS]  Dense grey slightly silty sandy GRAVEL. Sand is fine to coarse. Gravel is subangular and subrounded fine and medium of limestone sandstone and calcite. [ESTUARINE DEPOSITS]	8.50	-9.55			
- 9.00 - 9.50	В	21						9 —	8.90m to 8.95m; firm dark brown clay.  Stiff greyish brown slightly sandy slightly gravelly CLAY. Sand is fine and medium. Grav is subangular fine of limestone and sandstone [ESTUARINE DEPOSITS]		-10.05			
- 9.50 - 10.00 9.50 - 9.95 9.50 - 9.95	B D SPT	23 22	N = 45 (S)					-	Firm and stiff brown silty CLAY. [ESTUARINE DEPOSITS]	9.50	-10.55	 		
-10.00 - 10.50	В	24						-	Continued next page	(1.00)		×		

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Con	tract Name	Dubl	in Po	rt MP	2_3F	FM		Loc	cation	ı ID			
-fuc	RO	Clier	nt			rt Co				3	FN	ЛF	3H-	s.	.06
			o Reference	F211				-,		٦-		<b>'</b> '	<b>-</b> 111	J	UU
•		_	rdinates (m)	_		80 N	7341	33.30	Ground Elevation (m Datum) -1.05	Sh	eet 3	of 6			
		Hole	Туре	Cabl	e Pei	cuss	ion to	o Rota	ary Coring	Sta	atus		Preli	minar	у
Depth	Samp	ling ar	d In Situ Testing	Co	ore R	ecove	ery		Strata Details					Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	(Т	Depth hickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
- 10.50 - 10.95 10.50 - 11.00 10.50 - 11.09 10.50 - 10.95	D B SPT	25 26	N = 16 (S)						Firm brown slightly sandy silty CLAY. [ESTUARINE DEPOSITS]		10.50	-11.55	X X X X X X X X X X X X X X X X X X X		
- - - - 11.50 - 12.00	UT	27	82/600 mm					-			(1.50)		X		
- -12.00 - 12.50 -	В	28						- 12 — - -	11.85m to 11.95m; gravelly.  Firm to stiff dark brown slightly sandy gravelly CLAY. Sand is fine to coarse. Gravel is angula and subangular fine and medium of limestone and sandstone. [PORT CLAY]	ar	12.00	-13.05	× × × × × × × × × × × × × × × × × × ×		
- 12.50 - 12.95 12.50 - 12.95	D SPT	29	N = 36 (S)					-	12.50m to 12.95m; stiff.		(1.50)				
-13.00 - 13.50 13.00 - 13.50	B D	30 31						13 —					* * * * * * * * * * * * * * * * * * *		
- 13.50 - 13.95 - 13.50 - 13.95 	D SPT	32	N = 48 (S)					-	Very stiff dark brown slightly sandy gravelly CLAY. Sand is fine to coarse. Gravel is subangular to rounded, locally elongated, fine to coarse of dark grey limestone, brown sandstone and metamorphic. [PORT CLAY]		13.50	-14.55	0 0 1 0 1 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0		
- -14.00 - 14.50 - -	В	33						14 —							
- - 14.50 - 14.85   14.50 - 14.85  -	D SPT	34	50/200 mm (S)					-	14.50m to 14.85m; slightly gravelly coarse sand.				0 0 1 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0		
- 15.00 - 15.50	В	35						_	Continued next page				* * * * * * * * * * * * * * * * * * * *		
Notes															

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Cont	tract Name	Dubl	in Po	rt MP	<sup>2</sup> _3F	-М		Location	on ID			
-fug	RU	Clier	nt	Dubl	in Po	rt Co	mpar	าง		3F	<b>M</b> _I	RH.	S.	.06
1			o Reference	F211				.,		J.	'''-	٠١١٠	0	00
•						80 N	7341	33.30	Ground Elevation (m Datum) -1.05	Sheet	4 of 6			
			Туре	-					ary Coring	Status		Preli	minaı	ry
Depth	Samp		d In Situ Testing			ecove			Strata Details			ı		ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thicknes (m)	Level (m Datum	Legend	Water Strike	Backfill / Installation
- - - - 15.50 - 15.74 15.50 - 15.74 - - - - - -	D SPT	36	50/95 mm (S)					- - - - - - - 16 —	15.50m to 15.74m; sandy gravel. Sand is medium to coarse. Gravel is subangular and subrounded fine to medium of limestone, siltstone sandstone and possible schist.	(3.50)				
- - - 16.50 - 16.84 16.50 - 17.00 16.50 - 16.84	D B SPT	38 39	50/185 mm (S)					-	16.50m to 17.00m; slightly gravelly.					
-17.00 - 17.30 - - 17.00 - 17.50 - 17.30 - 18.00	В	40		100	0	0		17 —	Very dense dark grey to brownish grey sandy clayey GRAVEL with low cobble content. Sand is fine to coarse. Gravel is subangular to subrounded of fine to coarse of dark grey limestone and brownish grey siltstone. Cobbles (<50mm x 80mm x 90mm) are subrounded and rounded of dark grey limestone [GLACIAL					
- - - - - 17.50 - 19.00				80	0	0			DEPOSITS].  Dark grey and brownish grey clayey gravelly COBBLES. Gravel is subangular to subrounder of medium to coarse of dark grey and grey limestone, brownish grey brown siltstone and green schist. Cobbles (<70mm x 80mm x 120mm) are subrounded of dark grey limestone with some calcite veins (<5mm) [GLACIAL DEPOSITS]		-18.55			
-								- - - - - 19 —	18.70m to 19.00m; assumed zone of core loss.	(1.97)				
- - - - 19.50 - 19.90 - - 19.00 - 20.50	В	42		60	0	0		-	19.30m to 19.47m; 1 No. subrounded cobble (<60mm x 90mm x 170mm) of very strong dark grey limestone with occasional subhorizontal calcite veins (<2mm). Fresh.  Dark grey to brownish grey sandy clayey GRAVEL with low cobble content. Sand is fine to coarse. Gravel is subangular to rounded, locally elongated, fine to coarse of limestone siltstone calcite and metamorphic. Cobbles (<60mm x 90mm x 170mm) are subrounded  Continued next page	(0.43)				

Print Date

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Conf	ract Name	Dubl	in Po	rt MF	2_3F	M		Locatio	n ID			
-fug	DΠ	Clier	nt	Dubl						3FI	Л Б	RH.	<b>S</b> .	വട
			o Reference	F211				.,		<b>0.</b> .	<b>'</b> '	J1 1	0	اک
•		_	dinates (m)	_		80 N	7341	33.30	Ground Elevation (m Datum) -1.05	Sheet 5	of 6			
			Туре	_					ary Coring	Status		Preli	mina	у
Depth	Samp	ling an	d In Situ Testing	Co	re R	ecove	ery		Strata Details					ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
								-	and rounded of very strong and strong dark \grey limestone [GLACIAL DEPOSITS]. Assumed zone of core loss	(0.60)				
-								- - - -	Dark grey to brownish grey sandy clayey GRAVEL. Gravel is subangular to rounded, locally elongated, of fine to coarse of limestone siltstone calcite and metamorphic. [GLACIAL DEPOSITS]  Assumed zone of core loss.	(0.43)	-21.55 -21.98			
20.50 - 22.00				29	0	0		21 —	Assumed zone of core loss.					
-								- - 22 —		(1.82)				
_ 22.00 - 22.75 _				0	0	0		- - -						
22.75 - 23.15	В	43		54	0	0		23 —	Dark grey to brownish grey slightly clayey GRAVEL. Gravel is subangular to rounded, locally elongated, fine to coarse of limestone siltstone, calcite and metamorphic, possibly schist. [GLACIAL DEPOSITS].  Assumed zone of core loss	(0.40)	-23.80 -24.20			
- - - - 23.50 - 24.25				0	0	0		- - - - - 24 —		(1.10)				
24.25 - 24.65	В	44		100	0	0		- - - -	Dark grey to brownish grey slightly clayey GRAVEL. Gravel is subangular to rounded of fine to coarse of limestone siltstone and metamorphic, possible schist. [GLACIAL DEPOSITS].	24.25	-25.30			
[ -								-	Continued next page	25.00	-26.05			
Notos											<u> </u>			
Notes														

Print Date

16/11/2022

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

		Cont	ract Name	Dubl	in Po	rt MP	2_3F	М		Location	n ID			
-fug	RO	Clier	nt	Dubl	in Po	rt Coı	mpar	ny		3FN	Л F	ЗΗ.	S-	06
		Fugr	o Reference	F211	210					<b>O</b>	··—·			
•			dinates (m)	E718	3621.	80 N7	7341	33.30	Ground Elevation (m Datum) -1.05	Sheet 6	of 6			
		Hole	Туре	Cabl	e Per	cussi	on to	Rota	ary Coring	Status		Preli	minar	у
Depth	Sampl	ing an	d In Situ Testing	Co	re R	ecove	ery		Strata Details			·	Grour	ndwater
(m)	Туре	No.	Test Results	TCR (%)	SCR (%)	RQD (%)	FI (-)	Depth (m)	Strata Descriptions	Depth (Thickness) (m)	Level (m Datum)	Legend	Water Strike	Backfill / Installation
25.00 - 25.15									Strong to very strong locally medium strong					
25.15 - 25.38							9	- - - -	dark grey locally mottled grey LIMESTONE. Fresh to slightly weathered, locally distinctly weathered. Discontinuities, Set#1; bedding plane fractures, inclined (0-30°), very closely to medium spaced, planar and undulating, smooth and rough, very tight to tight, clean and with clay (<2mm) and calcite infill (<5mm) some to					
25.57 - 25.72	С	45	150 mm					-	frequent clay smears. Set#2, joints, inclined					
25.51 - 25.90 25.00 - 26.50				100	47	33	20	26 —	(60-90°), planar, undulating and rough, very tight and tight, clean. [LIMESTONE BEDROCK] 25.00m to 25.15m; non-intact. Recovered as angular and subangular medium and coarse gravel and cobble-sized fragments. 25.38m to 25.51m; moderately weak. Distinctly weathered to destructured with angular and subangular fine to coarse gravel-sized fragments, locally moderately weathered with inclined (70-80°) calcite veins (<8mm). 25.50m to 25.73m; inclined (70-80°) calcite vein (<10mm).					
26.30 - 26.50							9	-	25.84m to 25.90m; inclined (50°) calcite vein (<20mm). 25.90m to 26.30m; moderately weak. Distinctly					
								-	weathered to destructured with angular and subangular fine to coarse gravel-sized fragments					
26.60 - 26.70							-	-	and locally clayey. 26.30m to 26.50m; moderately weak. Moderately					
26.70 - 26.80 26.80 - 26.85							2	-	weathered. 26.50m to 26.60m; assumed zone of core loss.					
20.00								-	26.60m to 26.70m; non-intact. Recovered a coarse gravel to cobble-sized fragments.	(3.80)		$\Box$		
26.85 - 27.28 26.50 - 28.00				100	00 74	55	5	27 — -	26.80m to 26.85m; non-intact. Recovered as angular fine to coarse gravel-sized fragments. 26.85m to 27.28m; inclined (0.10°) drilling induced fractures associated with inclined (85-90°) calcite vein. 27.00m to 27.28m; incline (85-90°) calcite vein.					
27.28 - 27.35 27.35 - 27.43							9		(<5mm).			ш		
27.43 - 27.48 27.81 - 28.00	С	46	190 mm				-		27.28m to 27.35m; non-intact. Recovered as clayey angular to subangular fine and medium gravel-sized fragments. 27.43m to 27.48m; non-intact.					
27.48 - 28.80							9	-	bedding plane fractures associated with inclined (60-90°) joint. 28.00m to 28.50m; some inclined (70-80°) calcite veins (<3mm).					
28.00 - 28.80				100	89	43		-	, ,					
28.57 - 28.80	С	47	230 mm					29 — - - - -	End of Borehole at 28.80 m	28.80	-29.85			
- Notes Abbreviations	and res	sults da	ta defined on 'Note:	s on E	Explora	atory F	Positio	- - - on Re	cords'					

Print Date

		Contract Na	ame		Dublin Port MP2_3FM					Locati		
Tugi	RO	Client			Dublin Port Company					<b>∃3</b> F	MBH	<b> -S-06</b>
		Fugro Refe	rence	;	F211210						_	
•		Coordinate	s (m)		E718621.80 N734133.30	Ground	Elevation (m	Datum) -1.	05	Sheet		
		Hole Type			Cable Percussion and Rotary	Coring	]			Status	Pro	eliminary
					Standard Penetra	tion Te	st Results					
Test Depth (m)	Test Typ	e Self Wei	ight n (mm)	Test F	Result		Total Penetration (mm)	Hammer Serial Number	Energy	Ratio (%)	Casing Depth (m)	Water Depth (m)
0.50	S	0			(0,0/0,0,0,0)		450	AR3214		54	0.50	0.00
2.50	S	0			(0,0/0,0,0,0)		450	AR3214		54	2.50	0.00
4.50 6.50	S S	0			(0,0/0,0,0,0) (0,0/0,0,0,0)		450 450	AR3214 AR3214		54 54	4.50 6.50	0.00 0.00
7.50	S	0			(7,12/11,11,13,11)		450 450	AR3214 AR3214		54 54	7.50	0.00
8.50	S	0			(8,13/7,8,8,12)		450	AR3214		54 54	8.50	0.00
9.50	S	l ő			(3,5/8,10,11,16)		450	AR3214		54	9.50	0.00
10.50	S	0			(3,2/3,5,3,5)		450	AR3214		54	10.50	0.00
12.50	S	0		N=36	(3,4/7,7,10,12)		450	AR3214	5	54	12.50	0.00
13.50	S	0			(6,6/8,10,12,18)		450	AR3214	5	54	13.50	0.00
14.50	S	0			(8,10/50 for 200mm)		350	AR3214		54	14.50	0.00
15.50	S	0			(25 for 140mm/50 for 95mm)		235	AR3214		54	15.50	0.00
16.50	S	0		N=50	(10,12/50 for 185mm)		335	AR3214	5	54	16.50	0.00
											1	ĺ

	In Situ	Vane Test Res	ults	In Situ Hand Per	netrometer Results	Volatile Headspace Testing	g by Photoionisation Detector
Test Depth (m)	Test Type	Undisturbed Undrained Shear Strength (kPa)	Residual Undrained Shear Strength (kPa)	Test Depth (m)	Undisturbed Undrained Shear Strength (kPa)	Test Depth (m)	PID Result (ppm)

- Abbreviations and results data defined on 'Notes on Exploratory Position Records'

Template: FGSL/HBSI/FGSL SPT Summary.hbt/Config Fugro Rev5/18/02/2019/TS

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